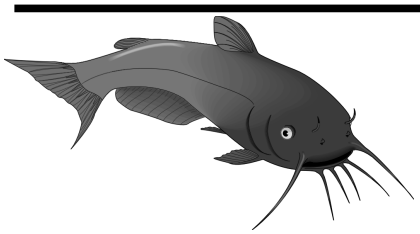
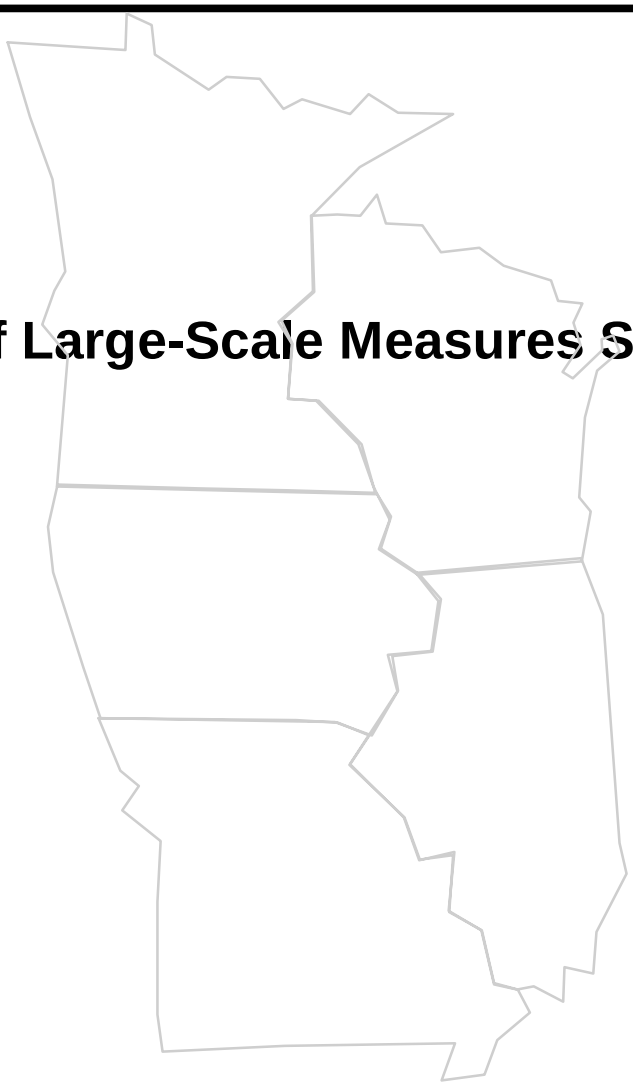
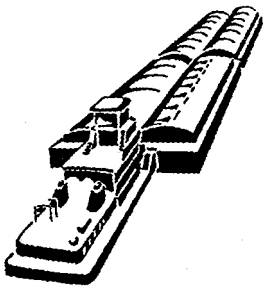
Upper Mississippi River – Illinois Waterway System Navigation Study





Summary of Large-Scale Measures Screening



INTERIM REPORT

January 2004

US Army Corps of Engineers

Rock Island District
St. Louis District
St. Paul District

EXECUTIVE SUMMARY

The Upper Mississippi River-Illinois Waterway System Navigation Study is a system feasibility study addressing navigation improvement planning and ecosystem restoration opportunities for the Upper Mississippi River and Illinois Waterway (UMR-IWW) System over a 50-year planning period. The study started in 1993 following a Reconnaissance-level report in 1991.

The principal navigation problem addressed by this study is the potential for significant traffic delays on the UMR-IWW Navigation System within the 50-year planning horizon. The principal environmental problems addressed by this study are changes to ecosystem structure and function imposed by the operation and maintenance of the existing 9-Foot Channel Navigation Project, and potential navigation system improvements. The primary opportunities are to reduce or eliminate commercial traffic delays and improve the national and regional economic conditions while restoring, protecting, and enhancing the environment. The goal of the System Study is to outline an integrated plan to ensure the economic and environmental sustainability of the UMR-IWW Navigation System.

The goal of this report is to summarize the identification and screening of the large-scale measures. However, the final product of the Navigation Study is the feasibility report, which will constitute the decision document for processing to Congress. Large-scale measures are navigation improvements involving extending the existing lock or providing a second lock at an existing lock and dam.

The process first identified a universe of potential large-scale measures that might improve system efficiency. This universe included evaluating 16 potential lock sites (Locks 11-25 on the UMR and Peoria and La Grange Locks on the IWW), six alternative placement locations for the construction of a new lock at each of these existing lock and dam sites, four different design types, and a variety of lock sizes. From this initial list, a two-part screening process was used to select the most promising measures.

The table on the following page shows the remaining lock locations and types after initial and secondary screening contained in this document. The only remaining size is the 1,200-foot by 110-foot lock. At most lock sites, the remaining alternatives include an option that provides for just one 1,200-foot lock, extending the existing lock (Location 2), and one option that would result in a new 1,200-foot lock in addition to the existing 600-foot lock. In most cases, any new lock would be placed in the auxiliary gate bay (Location 3). Exceptions include Lock 14 where a Location 3 lock was not viable; Locks 17 and 25 where new locks landward of the existing lock (Location 1) were carried through secondary screening as well; and Lock 19 where a 1,200-foot is already in place in Location 2. At the IWW locks, Location 1 locks appeared to be the preferred options, but Location 2 locks were also carried through the secondary screening.

REMAINING LOCK LOCATIONS AND TYPES FOLLOWING THE SECONDARY SCREENING						
Lock and Dam Site	**Location Number and Viable Types**					
	1	**2**	**3[3]**	**4[3]**	**5**	**6**
L/D 11	-	R	C	-	-	-
L/D 12	-	R	C	-	-	-
L/D 13	-	R	C	-	-	-
L/D 14[1]	-	R	-	C	-	-
L/D 15[1]	-	R	B	-	-	-
L/D 16	-	R	C	-	-	-
L/D 17	C	R	C	-	-	-
L/D 18	-	R	C	-	-	-
L/D 19[1]	-	-	B	-	-	-
L/D 20[2]	-	R	B	-	-	-
L/D 21	-	R	C	-	-	-
L/D 22[1]	-	R	C	-	-	-
L/D 24[2]	-	R	C	-	-	-
L/D 25	C	R	C	-	-	-
Peoria	C	R	-	-	-	-
La Grange	C	R	-	-	-	-

[1] These sites have rock foundations. All others (except for note 2) are sand-founded sites (requiring piles).

[2] These sites have mixed foundations; some locations would be rock-founded and some pile-founded.

[3] While only a Type B or C lock is shown, costs and performance evaluations to date have shown these lock options to be essentially equal. As a result, only the lower cost option is shown in the table, but if the system economic analysis shows justification for Location 3 or 4 locks, Types B, C, and possible hybrids of the two will be considered during the site-specific detailed design efforts.

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SECTION 1 - SUMMARY AND BACKGROUND

PURPOSE

The goal of this report is to summarize the identification and screening of the large-scale measures. The large-scale analysis began with the development of an array of new locks that were possible from an engineering standpoint. This array included evaluating 16 potential lock sites (Locks 11-25 on the UMR and Peoria and La Grange Locks on the IWW), six alternative placement locations for the construction of a new lock at each of these existing lock and dam sites, four different lock design types, and a variety of lock sizes. In addition, wicket dams, which allow tows to navigate over the dams in periods of high water, were considered at UMR Locks 17 and 20.

This report is organized into four general sections. Section 1 provides the background information on the study purpose and scope, definition and role of large-scale measures, and overview of the lockage process. The second section presents the process and results of the initial screening. The third section provides information from efforts to further quantify the benefits and costs of the measures surviving the initial screening. Finally, the report concludes with a secondary screening used to narrow the potential measures.

SUMMARY

Given the vast number of potential measures, it was critical to screen them down to a more manageable level prior to developing alternative plans. To accomplish this screening, a multi-step process ensued. Due to time and funding constraints, developing detailed information on the measures and screening them was essentially a parallel process, with screening occurring as adequate information became available. The value of this analytical process, which continually screens out the least promising measures, is that study resources are continually concentrated on those items showing the greatest promise.

The huge number of potential combinations of measures made it necessary to start screening locks by categories (location, size, and type). However, during this step, the input of various disciplines (environmental, economics, engineering, and plan formulation) was sought and considered. While efforts were made to consider site-specific conditions associated with each lock, this analysis was primarily conducted at the system level. Many of the site details will be looked at in greater detail if locks show justification as a result of this system study. First, a primarily qualitative screening took place to eliminate any lock locations that obviously had fatal flaws or were clearly dominated by another location. Similarly, various possible lock sizes (lengths and widths) were evaluated to select the best for further consideration. This screening resulted in the finding that the two most effective chamber sizes were 110 feet by 1,200 feet and 110 feet by 600 feet. As additional qualitative cost and construction impact information became available, it was also possible to eliminate some of the potential lock design types. Based on the limited benefits and the potential for severe environmental impacts, navigable wicket dam options at UMR Locks 17 and 20 were eliminated.

After these initial screening efforts, additional quantitative information was developed to allow further screening that would consider the total life-cycle costs, construction impacts, and time savings of each measure. Some of this information proved to be eliminating factors for a measure. However, for most of the locks surviving the initial location, size, and type screenings, the detailed cost, benefit, and impact information was quantified for use in further evaluations.

Following the further quantification of benefits and costs, a secondary screening was conducted to evaluate the cost effectiveness of the various measures. While cost effectiveness provided the framework, the screening also involved as assessment by the various technical disciplines and consideration of qualitative data. Results of the secondary screening showed that, for the UMR sites, the development of a low-cost alternative lock extension (Location 2 Type R) resulted in elimination of other lock extension options and new 600-foot chamber options. In addition, a measure was identified at each site that would provide a new 1,200-foot lock in addition to the existing 600-foot lock. In most instances, this was a new lock in the existing auxiliary gate bay (Location 3); however, at sites where a new lock landward of the existing lock (Location 1) is an option, it was carried through secondary screening for further consideration as well. More severe impacts to navigation are anticipated at IWW sites due to year-round navigation and the associated lack of a winter closure period to construct improvements. At these sites, a new 1,200-foot lock landward of the existing lock (Location 1) appears to be preferred to avoid these impacts, but the 1,200-foot lock extension was carried through secondary screening as well.

Major steps and products in the large-scale measure development and screening process included: developing the universe of large-scale measures and qualitative screening summarized in the *Large Scale Measures of Reducing Traffic Congestion Location Screening* report (July 1999); *Large Scale Measures of Reducing Traffic Congestion: Conceptual Lock Designs* report (July 1996); *Interim Revised Lock Extensions Design Concepts* (July 2000); and the summary of the screening of large-scale measures included in this report. The focus of this report is to document and summarize initial and secondary large-scale screening processes.

BACKGROUND

Large-Scale Measure Definition and Role

Large-scale measures include improvements that involve extending the existing lock or providing a second lock at an existing lock and dam site. An additional measure—providing a wicket dam to allow navigation to pass over the dam—was considered at lock and dam sites that have a substantial period of uncontrolled flow. Uncontrolled flow occurs when the dam gates are raised clear of the water during periods of high river flow. Of the sites in the study, Locks and Dams 17 and 20 are those with the greatest duration of uncontrolled flow (besides Peoria and La Grange Locks and Dams that already have navigable dams).

The navigation time savings of large-scale measures are significantly greater than those for small-scale, however, the costs are also significantly higher. A 1,200-foot lock (corresponding to the maximum tow size presently used on the UMR-IWW system) would obviate the need for a double lockage, allowing a tow to transit through the lock in a single lockage. Providing an additional chamber of smaller size than 1,200 feet (e.g., 600 feet) would still require double lockages. However, this second chamber would reduce delays by allowing some traffic to be processed in the small lock.

Steps in the Lockage Process

Where the word “performance” is used, it will generally refer to the lock’s capability to perform its basic function of locking boats. Varying levels of performance of the lock are represented by the several “types” of lock designs. The focus of the large-scale measures is on improving overall lock efficiency by reducing the number of steps in the lockage process (1,200-foot chambers, wicket

gates) or by providing an additional lock.

Most Mississippi River and Illinois Waterway locks are 600 feet long by 110 feet wide. Tows that are this size or smaller are able to lock through as a single lockage or in one piece. Larger tows, such as the prevailing 15-barge tow size that is nearly 1,200 feet long by 105 feet wide, must lock through as a double lockage or in two pieces. The double lockage adds several steps to the lockage process as well as considerable time. During this multi-step process, the tow sends half its barges through the lock at a time, tying up the lock while it disconnects the two sets of barges, sends each half through separately, and then reconnects and moves out of the way. At 600-foot chambers, double lockages typically take between 80 minutes and 2 hours, compared to a range of 20 minutes to 40 minutes for single lockages. Considerable variability in lockage times occurs due to differences in lockage types, lock sites, river and weather conditions, and crew and boat factors.

The major elements of double and single lockages, the two most common types of lockage, are summarized below. For these lockages, the total lockage time equals the sum of the duration of each lockage step. Figures 1 and 2 compare a typical double lockage process with a single lockage process, which would occur if a 1,200-foot chamber were available (note: some steps have been consolidated for simplification).

Double Lockage Steps:

(1) Approach the lock
(2) Enter the chamber
(3) Uncouple the tow and back the powered cut out of chamber*
(4) Close lock gates
(5) Fill or empty the lock chamber
(6) Open lock gates
(7) First cut exits lock chamber (with tow haulage assist)*
(8) Close lock gates*
(9) Fill or empty the lock chamber {opposite direction of step (5)}*
(10) Open lock gates*
(11) Powered/second cut enters the lock chamber*
(12) Close lock gates*
(13) Fill or empty the lock chamber*
(14) Open lock gates*
(15) Recouple the tow (powered cut to the unpowered cut previously locked through)*
(16) Exit the lock

*These steps are only involved with double lockages when the size of the tow exceeds the size of the lock chamber. The most common example of this in the current navigation system is with nearly 1,200-foot-long tows transiting through 600-foot-long locks.

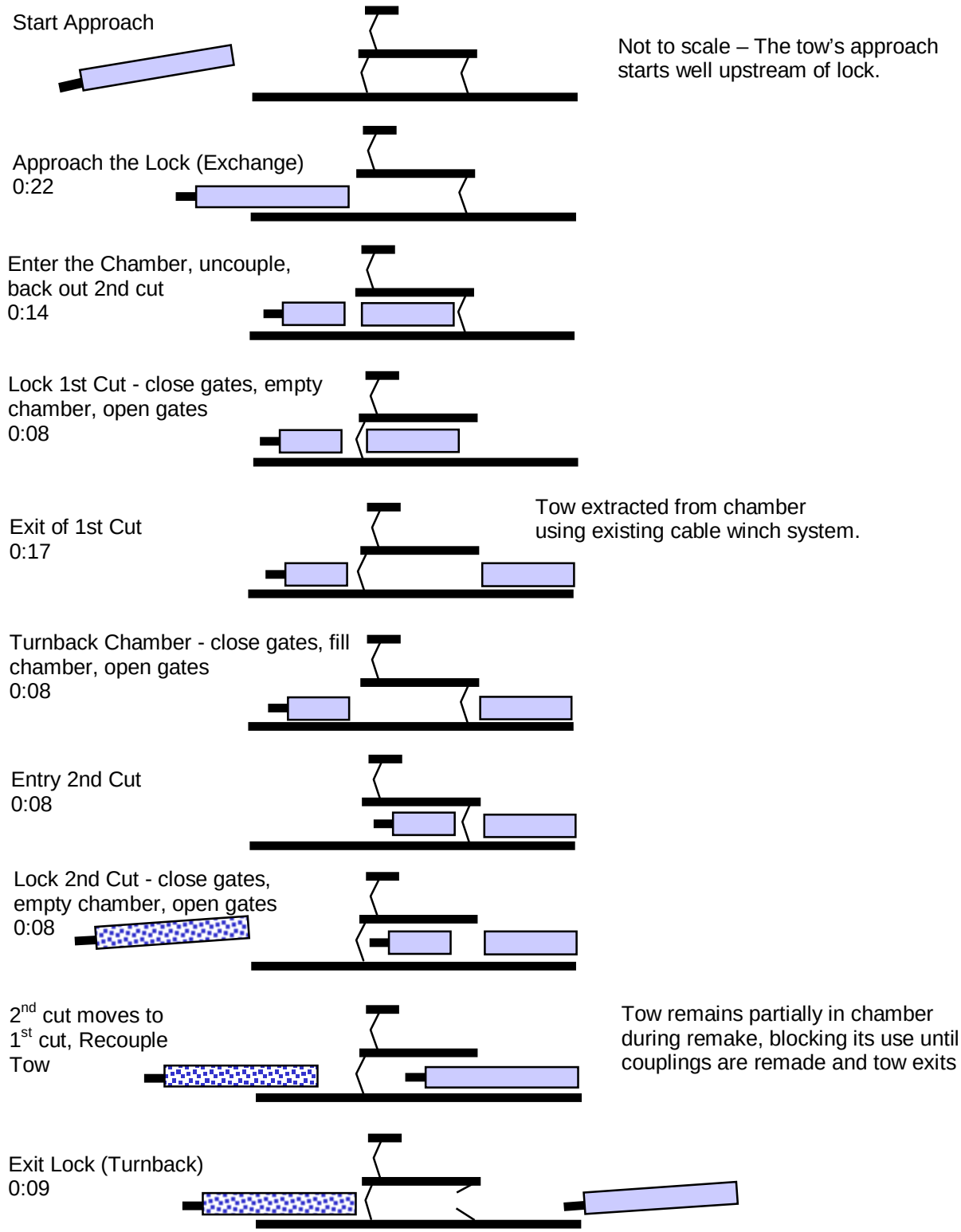
Double lockages require several steps. Vessels first approach the lock. The approach process includes the time for the tow to approach the lock, align with the guidewall, and place its bow (front of boat) over the sill of the chamber. For a vessel traveling downstream (downbound vessel), either before or during the approach the chamber is filled to the upper pool level and the upper gates opened. The vessel entry time extends from the time when the tow gets its bow over the sill until the tow is fully in the chamber and the lock gate can be closed. However, due to the tow's length, a first cut (unpowered section of tow) must be uncoupled from the front end of the

tow and locked separately. These cuts generally consisting of nine barges that fill the whole lock. Once the cut is secured inside the lock chamber, the towboat and remaining barges then back away and the upstream lock gates are closed. The vessel is then lowered by closing the filling valves and opening the downstream, or emptying, valves. The water in the chamber flows back into the culvert and then out into the lower pool, lowering the water level in the chamber until it is equal to the water level downstream of the lock.

Once the first cut is at the lower pool elevation, the gates are opened and some form of assistance (a tow haulage winch or a switch boat) must pull, or extract, the first cut from the chamber. The cut is typically pulled out along the guidewall and tied off to wait for the second cut. When the first cut is clear of the gates, the gates are closed and the chamber is turned back, filling to get the chamber back to the upper pool elevation of the second, or powered, cut. As soon as the upper gates are opened, the second cut can enter the chamber and be locked through as a single. The one difference is that before the tow can exit the lock facility it must move forward to the first cut and remake its couplings. At sites with 600-foot or shorter guidewalls, the second cut usually remains partially inside the chamber while the first cut is along the guidewall, which temporarily eliminates the ability to use the chamber to lock the next tow. The process is reversed for tows going upstream (upbound vessels).

> Single Lockage Steps: Only possible when lock chamber is at least as large as the tow (e.g., 600-foot or less tows at existing 600-foot facilities and 1,200 feet or less tows at 1,200-foot lock sites)
>
> (1) Approach the lock
> (2) Enter the chamber
> (3) Close lock gates
> (4) Fill or empty the lock chamber
> (5) Open lock gates
> (6) Exit the lock

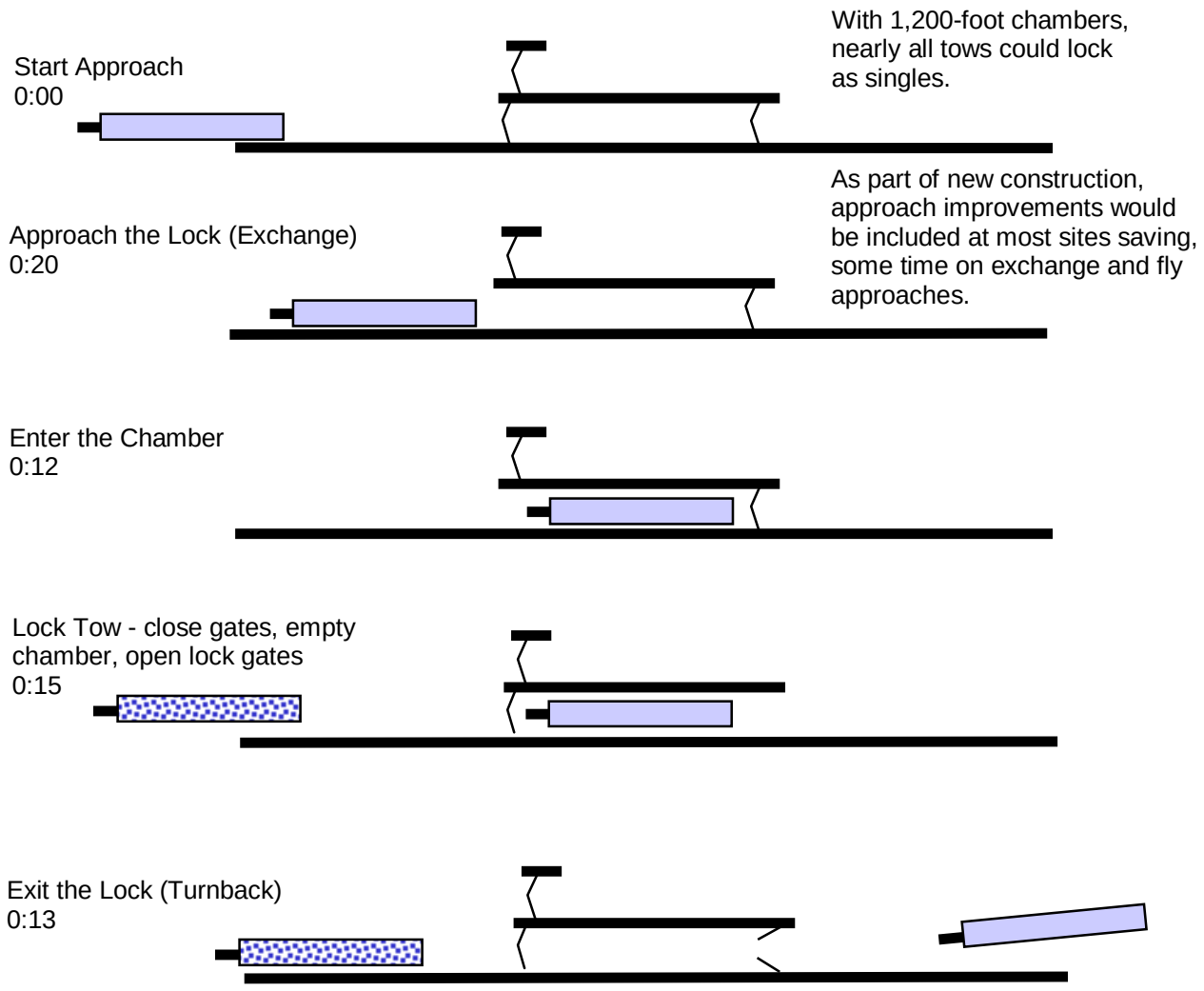
Single lockages follow the same general process but eliminate several steps. Just like a double lockage, the full tow approaches and enters the chamber. However, since the tow is the same size as the lock chamber, there is no need to break couplings and lock a portion of the tow separately. As a result, there is no need to break and remake couplings or turn back the chamber for a second cut. Eliminating these steps saves considerable time.



Total Lockage Time 1:48

Note: Approx. lockage time in hour:minutes by step. Diagram shows an exchange approach followed by a turnback lockage.

FIGURE 1: Double Lockage Elements Downbound Lockage at Existing 600-Foot Lock.



Total Lockage Time 0:60. Approximate Time Savings vs. Existing Double Lockage at 600-foot Lock is 45 to 50 minutes.
Note: Approx. lockage time in hour**:**minutes by step. Diagram shows an exchange approach followed by a turnback lockage.

FIGURE 2: Lockage Elements Downbound with Extended 1,200-Foot Lock.

SECTION 2 - INITIAL SCREENING

IDENTIFICATION OF MEASURES

In the Upper Mississippi River and Illinois Waterway Navigation Study Reconnaissance Reports completed in June 1991 and October 1990, respectively, 16 lock and dam sites were identified as having potential for new lock construction within the 50-year planning horizon. These sites included Peoria and La Grange Locks and Dams on the Illinois Waterway, and Locks and Dams 11-25 on the Mississippi River (there is no Lock and Dam 23 and Lock 19 is a 1200 foot long chamber) and were the focus of this feasibility study effort. Once it was determined that six locations would be considered at each site with as many as four design types at each location, the universe of large-scale measures was large. With such an enormous design task, a screening process was needed to identify only the most promising locations and design types.

The large-scale analysis began by developing an array of new locks that are possible from an engineering standpoint. The large-scale screening process followed the process of initially screening measures using a multi-disciplinary team and readily available information. After the least promising options were eliminated, the study team gathered more detailed, quantitative information to further analyze the remaining measures. For the initial evaluations, screening occurred separately for alternative lock locations, lock sizes, and lock types. The following sections have been organized to first present and explain the various lock locations, sizes, and types and then to review the available data and screening process used to narrow the choices.

Locations

Unless another meaning is obvious by the context, the use of “site” means any of the existing lock and dam sites included in the Navigation Study (e.g., L&D 20, L&D 24, Peoria Lock and Dam). This distinction is necessary to avoid confusion with lock “locations” defined below.

During the initial phases of the feasibility study, options such as replacing two lock and dam sites with a single site were considered. However, engineering efforts demonstrated that the existing structures could continue to perform with continued rehabilitation. This finding, along with environmental concerns and potential needs to acquire additional land and raise levees, resulted in the determination that it was unnecessary and uneconomical to build new dams along with new locks. This left any potential lock placements to be made at the existing lock and dam sites. Six possible “locations” at each lock and dam were considered (see Figure 3). With these six locations, any placement from overbank to overbank was possible; however, practical matters restricted some locations at particular sites. During the screening process, some of the six locations proved to be infeasible. The locations considered were as follows:

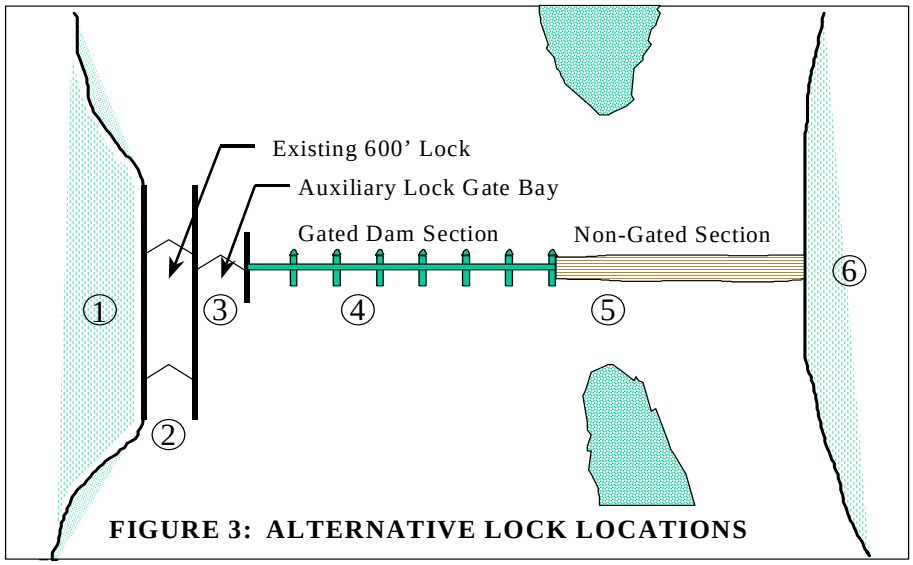


FIGURE 3: ALTERNATIVE LOCK LOCATIONS

Location 1 is landward of the existing lock. For most sites, Location 1 would entail land-based construction techniques as opposed to marine-based construction required at most other locations. Large quantities of soil and/or rock excavation would be required at this location. Location 1 typically would require relocations of railroad track, utilities, private property, and/or other infrastructure. For the most part, construction would not interfere with navigation access to the existing lock. A considerable amount of channel work would be required to allow tows to reach a Location 1 lock safely. Unlike locks at other locations, a Location 1 lock would disrupt access to the existing lock for materials and personnel. However, easier access (for personnel and supplies) would be available to the new lock site than locks at other locations.

Location 2 is an extension of the existing 600-foot-long lock to result in a 1,200-foot-long lock. The only viable designs are those that could be constructed while maintaining navigation with only minimal traffic interruptions and low accident risks. The lock extensions would generally be made downstream to work in shallower water. Also, the upper gate sills of the existing locks are too high to function as lower gate sills without modification. Little or no channel work would be required at this location.

Location 3 utilizes the existing auxiliary lock miter gate bay (if present) to construct the new lock. This location would also require navigation interruptions during construction. However, it typically would require little or no channel work due to its close proximity to the existing channel. It also would make use of the existing intermediate lockwall and the existing auxiliary lock miter gate bay.

Location 4 is through the gated section of the dam. Although any placement through the gated section is possible, placement toward the existing lock side is generally preferred because less channel work is required and the lock would be more accessible to lock personnel. Siting a lock at Location 4 eliminates one or more of the existing dam gates. This loss of flow capacity would impact upstream water surface elevations. For the system study, it has been assumed that any dam gates removed would be replaced one-for-one at available space along the axis of the dam

(where there presently is lesser or no flow capacity). At flood stages and after navigation has ceased, providing flow through the new lock chamber (controlled by an upstream lift gate) is a possibility as well.

Location 5 places the lock in the non-overflow or overflow sections of the dam (if present) beyond the gated section of the dam. Again, the preferred placement would be toward the existing lock side of the non-overflow or overflow sections. At most lock sites, a Location 5 lock would require extensive channel work and have adverse environmental impacts. In addition, it would require greater utility modifications and offer poor accessibility for operating personnel. Nevertheless, a Location 5 lock design was developed to determine its first cost for comparison with other locations.

Location 6 is a land-based location on the opposite bank from the existing lock. To a greater extent than at Location 5, Location 6 locks would require extensive channel work and have adverse environmental impacts. An additional problem at Location 6, resulting from the fact that there would be no flow adjacent to the new lock (unless additional dam gates were added), is that siltation would be a problem requiring frequent dredging work. No conceptual lock designs for Location 6 were developed because the concurrent location screening investigation determined that the added problems of this location were severe enough to eliminate all Location 6 locks. Nevertheless, the basic cost of a Location 6 lock would be expected to be similar to a Location 1 lock, the other land-based location.

Sizes

Various lock sizes were evaluated that varied both in length and width. The evaluated sizes included 200-foot increments from 200 feet to 1,200 feet for lock length, as well as widths of 110 feet to 220 feet. Based on further system constraints related to the channel as well as downstream locks, these sites represent a full range of options.

Types

As noted earlier, four conceptual lock design types were considered to provide an array of measures with varying cost and performance. These ranged from traditional lock construction (with somewhat higher performance) to locks of low first cost (with reductions in performance). Innovative designs were also of central importance.

Cost savings was a paramount goal in the design approach for developing the lock concepts. To help achieve this goal, all past design criteria and construction standards were open for reevaluation. Several innovations were explored and developed by the three districts working on the Navigation Study. Most of the innovations were estimated to provide substantial cost savings compared to traditional lock construction. The study team also recognized the need to develop more environmentally friendly designs. Several design measures were included to avoid and minimize environmental impacts compared to traditional lock construction. These include use of less construction material through design innovations, beneficial use of dredged material, improved approach conditions to reduce tow maneuvering, designs of shorter construction duration, and construction without cofferdams. The potential locations for staging and disposal also were evaluated to help minimize site-specific environmental impacts.

Although there are site- and location-specific differences, the lock types evaluated are generally defined as follows:

Type A. A "Type A" conceptual lock design is a lock and guidewalls designed according to current design standards and traditional construction methods. The lock would be constructed within a dewatered cofferdam. The guidewalls would follow recent in-the-wet construction methods of beams spanning between cellular structures. The lock would typically have concrete gravity or U-frame walls, a side port filling and emptying system, and a downstream miter gate and either an upstream miter gate or a lift gate. A Type A lock would have the highest performance levels but also the highest first cost. Construction risks would be low for this type of lock.

Type B. A "Type B" conceptual lock design is a lower cost lock and guidewalls utilizing construction techniques proven in marine construction that heretofore have not commonly been used in lock construction. Use of these construction techniques would be innovative in the lock construction arena. A Type B lock would have a reduction in performance and a possible reduction in durability compared to a Type A lock. A Type B lock would present moderate to high risk to construct.

Type C. A "Type C" conceptual lock and guidewalls design is a lower first cost design that still is safe with predictable performance. This lock type could be less durable and possibly perform lower than Types A and B locks. To accomplish the cost savings, certain design standards were relaxed, with resulting tradeoffs in performance. The Type C conceptual designs include innovative in-the-wet construction techniques. A Type C lock would typically present low to moderate risks to construct.

Type R. A "Type R" lock was developed in 1998 as a refinement of the Type C lock. This writing develops only Type 2R, the extension of the lock to 1,200 feet. However, some similar types of savings could be identified for other locations as well. See the Engineering Appendix and Addendums for documentation of Type 3R and screening of other Location 3 types. Type 2R is a lowest first cost design that still is safe with predictable performance but with additional tradeoffs on performance. However, additional design modifications were included to reduce the impacts to navigation during construction and further lower construction costs. These design features include: not extending the filling and emptying culverts into the extension, utilizing the existing lower guidewall by strengthening it for use as the landside lockwall, maintaining the same sill and floor depth as the existing lock (resulting in a sill 1.38 times draft at minimum tailwater versus 2 or 1.7 times draft considered for Types A-C), using float-in lower gate monoliths, minimal approach/channel work, and only constructing a 600-foot downstream guidewall.

Initial Quantification of Costs

This section generally represents a preliminary qualitative screening. However, as quantitative information became available it was considered in the screening process. This overlap between quantitative and qualitative efforts resulted from the condensed study time frame, which required concurrent efforts in many areas. The following are the first cost estimates available by the end of the initial screening process. These costs were developed as part of the *Large Scale Measures of Reducing Traffic Congestion: Conceptual Lock Designs* report (July 1996). However, this quantification process was influenced by preliminary information from the qualitative assessment, as evidenced by the decision not to conduct detailed quantification of the costs of Location 6 locks or Type A locks at Locations 2 and 3.

The existing locks and dams on the Mississippi River and Illinois Waterway are either rock-founded or pile-founded. The new lock design concepts vary depending on their type of foundation. The resulting concepts are only typical of what could be done. Endless variation in

design detail is possible, but the objective in the system phase of the study was to establish engineering feasibility rather than to optimize the various design elements. If the system study demonstrates justification lock construction, the designs will be further evaluated and analyzed on a site-specific basis.

Table 1 summarizes cost estimates for the rock-founded 1,200-foot and 600-foot locks. Table 2 shows cost estimates for the pile-founded locks. The *Conceptual Lock Designs* report contains backup cost estimates from which the summarized cost estimates were derived. Since the scope of the present investigation is very broad, these estimates are not absolute; however, they are useful for comparison and screening purposes. The reader will note that a 600-foot-long lock is considerably more costly than 50 percent of a 1,200-foot-long lock because the guidewall requirements, miter gate bays, and much of the lockwalls are the same for both lock sizes. In addition, there are other common costs for construction mobilization, overhead, and dewatering. The additional features to expand from a 600-foot lock to a 1,200-foot lock are chamber-type monoliths (lockwalls). These are the least expensive lock monoliths to construct.

TABLE 1: ROCK-FOUNDED 1,200-FOOT AND 600-FOOT LOCKS			
COMPARISON OF SUMMARIZED COSTS ($1,000's, 1996 Price Level)[1]			
LOCK LOCATION AND TYPE[2]	BASIC LOCK COST[3]	OTHER FIRST COSTS[4]	TOTALS
LOCATION 1 - 1200'			
TYPE A	$ 188,000	$ 18,000	$ 206,000
TYPE B	176,000	19,000	195,000
TYPE C	171,000	20,000	191,000
LOCATION 2 - 1200'			
TYPE B	115,000	5,000	120,000
TYPE C	110,000	5,000	115,000
LOCATION 3 - 1200'			
TYPE B	131,000	6,000	137,000
TYPE C	122,000	5,000	127,000
LOCATION 4 - 1200'			
TYPE A	206,000	7,000	213,000
TYPE B	184,000	7,000	191,000
TYPE C	165,000	7,000	172,000
LOCATION 5 - 1200'			
TYPE A	234,000	12,000	246,000
TYPE B	187,000	12,000	199,000
TYPE C	161,000	12,000	173,000
LOCATION 1 - 600'			
TYPE A	$ 168,000	$ 24,000	$ 192,000
TYPE B - 600'	152,000	24,000	176,000
TYPE C	147,000	25,000	172,000
LOCATION 3 - 600'			
TYPE B	109,000	5,000	114,000
TYPE C	96,000	5,000	101,000
LOCATION 4 - 600'			
TYPE A	176,000	7,000	183,000
TYPE B	163,000	7,000	170,000
TYPE C	154,000	7,000	161,000
LOCATION 5 - 600'			
TYPE A	195,000	12,000	207,000
TYPE B	159,000	12,000	171,000
TYPE C	146,000	12,000	158,000

NOTES:

[1] The cost estimates shown above are not comprehensive. They do not take into account economic impacts to navigation during construction, environmental impacts, and other site-specific costs and impacts.
[2] There are no Type A locks at Locations 2 and 3, and Location 6 was eliminated by the location screening effort. In addition, there already is a 600-foot lock at Location 2.
[3] The basic lock costs include the necessary dam modifications (mostly a consideration for Location 4 lock types), the 05 "Locks" account code, and the 05.60. "Guidewalls" account code. Contingencies; planning, engineering, and design; and construction management are included in these costs.
[4] The costs in this column include real estate costs, channel costs, and some of the other first costs that tend to be more site-specific.

TABLE 2: PILE-FOUNDED 1,200-FOOT AND 600-FOOT LOCKS			
COMPARISON OF SUMMARIZED COSTS ($1,000's, 1996 Price Level)[1]			
LOCK LOCATION AND TYPE[2]	BASIC LOCK COST[3]	OTHER FIRST COSTS[4]	TOTALS
LOCATION 1 - 1200'			
TYPE A	$ 326,000	$ 12,000	$ 338,000
TYPE B	219,000	12,000	231,000
TYPE C	163,000	12,000	175,000
LOCATION 2 - 1200'			
TYPE B	160,000	1,000	161,000
TYPE C	151,000	1,000	152,000
LOCATION 3 - 1200'			
TYPE B	200,000	2,000	202,000
TYPE C	194,000	2,000	196,000
LOCATION 4 - 1200'			
TYPE A	373,000	0	373,000
TYPE B	283,000	0	283,000
TYPE C	248,000	0	248,000
LOCATION 5 - 1200'			
TYPE A	342,000	178,000	520,000
TYPE B	246,000	186,000	432,000
TYPE C	227,000	186,000	413,000
LOCATION 1 - 600'			
TYPE A	$ 273,000	$ 15,000	$ 288,000
TYPE B	191,000	14,000	205,000
TYPE C	149,000	16,000	165,000
LOCATION 3 - 600'			
TYPE B	172,000	1,000	173,000
TYPE C	160,000	1,000	161,000
LOCATION 4 - 600'			
TYPE A	339,000	0	339,000
TYPE B	232,000	0	232,000
TYPE C	217,000	0	217,000
LOCATION 5 - 600'			
TYPE A	287,000	179,000	466,000
TYPE B	205,000	185,000	390,000
TYPE C	181,000	185,000	366,000

NOTES:
[1] The cost estimates shown above are not comprehensive. They do not take into account economic impacts to navigation during construction, environmental impacts, and other site-specific costs and impacts.
[2] There are no Type A locks at Locations 2 and 3, and Location 6 was eliminated by the location screening effort. In addition, there already is a 600-foot lock at Location 2.
[3] The basic lock costs include the necessary dam modifications (mostly a consideration for Location 4 lock types), the 05 "Locks" account code, and the 05.60. "Guidewalls" account code. Contingencies; planning, engineering, and design; and construction management are included in these costs.
[4] The costs in this column include real estate costs, channel costs, and some of the other first costs that tend to be more site-specific.

LOCK LOCATION SCREENING

Once the array of possible measures was identified, a qualitative screening took place to eliminate any alternative lock locations that obviously had fatal flaws or clearly were dominated by another location. Looking at the universe of large-scale measures, it was obvious that some lock placements had serious problems. To help narrow the choices, a qualitative screening process was

used, employing multi-disciplined teams. The location screening work occurred concurrently with the development of the lock concepts and is documented in an interim report entitled, *Large-Scale Measures of Reducing Traffic Congestion, Location Screening*. This effort resulted in recommending 43 of a potential 96 alternative lock locations under initial consideration (16 sites, times 6 locations) for further analysis.

As part of the screening, locations were evaluated based on environmental impacts, navigational concerns, operational concerns, civil and structural design concerns, real estate needs, and hydraulic concerns for both the construction and normal operating conditions. To complete the location screening, the multi-disciplined study teams used a multi-phased approach that is summarized below.

First, the team used available information for each lock and dam site, which included drawings, maps, navigation charts, photographs and individual knowledge of the area, to eliminate locations obviously unsuited for a new lock because of existing constraints that by observation alone made those locations undesirable. Examples of these obviously unsuitable locations include those requiring the relocation of an entire downtown business district or the relocation of a railroad penned in by a high bluff. All of the participating disciplines had equal weight, including construction, environmental, geotechnical, hydraulics, operations, real estate, and civil/structural design. Criteria used for this first screening were general in scope. Detailed criteria were developed as the screening process progressed. Table 3 shows the criteria grouped by discipline.

The second phase of the location screening involved site visits to each of the 16 lock and dam sites under consideration for new lock construction. The teams making the site visits included the environmental, hydraulics, and civil disciplines; Corps of Engineers lock personnel; and representatives of the U.S. Fish and Wildlife Service, State resource agencies, and the River Industry Action Committee (RIAC). On the site visits, each of the represented disciplines considered all that was observable in their area of expertise for each of the six alternative lock locations. Some of the observed items included:

- Physical advantages or constraints to construction of a new lock.
- Existing navigation conditions (both favorable conditions and conditions associated with time delays, hazards, or excessive tow maneuvering, such as outdraft conditions).
- Location of obvious environmentally significant resources relative to the potential construction area for a new lock and any channel changes.
- Access, safety, efficiency, and other operational concerns.

TABLE 3: CRITERIA FOR EVALUATING NEW LOCK LOCATIONS	
Discipline	Evaluation Criteria
CONSTRUCTION	-Land access to construction site
	-Water access to construction site
	-Existing navigation - impacts on work areas
	-Cofferdam constructibility
	-Project constructibility
ENVIRONMENTAL	-Existing Federal/State wildlife sanctuaries
	-Mitigation opportunities
	-Endangered species habitat
	-Existing habitat sites
	-Existing rare plant species
	-National Historic Register sites
	-Suspected historical sites
	-Archaeological sites
	-Suspected archaeological sites
	-Hazardous, toxic, and radioactive waste
	-Recreational activity/adjacent recreation areas
GEOTECHNICAL	-Geological profile of sites
	-Depth to sound rock
	-Rock/soil excavation limits
	-Major weak soil lenses
	-Anticipated rock/soil permeabilities
	-Anticipated soil stability problems
HYDRAULICS	-Existing channel alignment
	-Better location for channel
	-Locations of frequent channel maintenance (dredging)
	-Channel approach conditions
	-New channel requirements (wing dams, weirs, etc.)
	-Magnitude of excavation/dredging for new channel
	-Existing hydraulic constraints at lock and dam
	-Can gates be added to maintain existing flow capacity
	-Filling/emptying requirements (one or two channels)
OPERATIONS	-Access for operating personnel and equipment
	-Existing maneuvering problems at lock entrance/exit
	-Centralization/separation of operating personnel
	-Guidewall requirements
	-Maintenance of two channels (lock separation)
	-Ice flow characteristics
	-Land access for recreation boating and related activities
	-Safety concerns with expanded lock operations
REAL ESTATE	-Existing Government-owned property
	-Real estate needs
	-Extent of property development adjacent to lock and dam sites
CIVIL/STRUCTURAL	-Adjacent land topography
	-Required relocations (Hwy/RR/utilities/drainage)
	-Existing bridge restrictions on navigation channel
	-Disposal sites for maintenance dredging
	-Impacts to completed lock and dam rehabilitation work
	-Special needs to accommodate location
	-Construction sequencing and impacts on navigation
	-Impacts to existing lock and dam structure, stability, etc.
	-Compatibility with existing structures
	-Costs

This rating procedure was the final step in the initial location screening process. Based upon all the preceding information gathered, study team members of each discipline rated each location at each lock and dam site. The scale ranged from 5 for an excellent location for new lock construction to 1 for a poor location. Table 4 shows the results of this rating process. The combined ratings are the summation of the ratings of all disciplines. Team members used the criteria of Table 4 in assessing the locations. In general, the three highest-rated locations were kept for further analysis unless any of their combined ratings was significantly lower than the highest 1 or 2 rated alternative(s), or there were some disqualifying factors. A fourth alternative was not eliminated if it rated nearly the same as the third highest. For example, Locks 16, 18, and 24 all had Location 5 locks rated similar to some of the remaining locations. However, this location was eliminated due to disqualifying factors associated with environmental impacts and operational concerns (relocating channels, etc.). Location 4 at Lock 11 was eliminated due to concerns with potential endangered species impacts and flow replacement. It was estimated that to bring these Location 4 and 5 locks to an acceptable level of navigability, the costs would be exorbitant and the environmental impacts would be excessive. Conversely, at Locks 24 and 25, Location 2 was retained due to minimal environmental and real estate impacts despite a lower overall rating than some of the other options. The locations that were least preferred by the group assessment were eliminated from further study. The relative ratings of the remaining locations should not be construed as indicating a ranking from best to worst locations. Rather, the higher-rated locations are merely those that survived the location screening process to be considered in the quantitative screening. The location screening helped to narrow the scope of work to the most promising locations to be considered in the quantitative screening work.

The screening reduced the number of alternative lock locations from 96 to 43 as shown in Table 5. These remaining locations were further evaluated and screened in subsequent quantitative studies discussed later in this report.

TABLE 4: LOCATION SCREENING RESULTS

LOCK SITE	LOCK LOCA-TIONS	Construction Ratings	Environmental Ratings	Geotechnical Ratings	Hydraulic Ratings	Operations Ratings	Real Estate Ratings	Civil Ratings	Structural Ratings	Combined Ratings	Disposition
11	1	3	3	4	1	1	1	1	4	18	Eliminated
	2	4	4	4	4	2	4	3	2	27	Survived
	3	4	4	4	4	4	4	4	2	30	Survived
	4	4	2	4	4	3	4	4	2	27	Eliminated
	5	5	X	4	1	1	3	2	4	20	Eliminated
	6	5	X	4	1	1	1	1	2	15	Eliminated
12	1	4	1	4	1	1	1	1	4	17	Eliminated
	2	4	4	4	3	2	4	3	2	26	Survived
	3	4	4	4	4	4	4	4	2	30	Survived
	4	4	2	4	2	3	4	4	2	25	Survived
	5	5	X	4	1	1	1	2	4	18	Eliminated
	6	5	X	4	1	1	1	1	2	15	Eliminated
13	1	5	X	4	1	1	4	1	4	20	Eliminated
	2	4	5	4	3	2	4	3	2	27	Survived
	3	4	3	4	4	4	4	4	2	29	Survived
	4	4	3	4	4	3	4	5	2	29	Survived
	5	5	X	4	1	1	4	2	4	21	Eliminated
	6	5	X	4	1	1	1	1	2	15	Eliminated
14	1	4	3	5	1	2	4	2	4	25	Eliminated
	2	4	5	5	3	2	4	3	2	28	Survived
	3	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	4	4	3	5	5	3	4	5	3	32	Survived
	5	5	X	4	1	2	5	1	4	22	Eliminated
	6	5	X	5	1	1	2	1	4	19	Eliminated
15	1	3	X	5	1	1	1	1	4	16	Eliminated
	2	4	5	5	3	2	3	4	2	28	Survived
	3	4	1	5	3	5	4	3	2	27	Survived
	4	4	1	5	1	2	4	1	1	19	Eliminated
	5	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	6	2	5	5	1	1	1	1	1	17	Eliminated
16	1	5	3	4	1	1	4	2	4	24	Eliminated
	2	4	5	4	1	2	4	3	2	25	Survived
	3	4	3	4	4	4	4	4	2	29	Survived
	4	4	3	4	5	3	4	5	2	30	Survived
	5	5	X	5	3	2	5	2	4	26	Eliminated
	6	5	X	5	1	1	1	1	4	18	Eliminated
17	1	5	2	4	3	1	4	4	4	27	Survived
	2	4	4	4	4	2	4	3	2	27	Survived
	3	4	4	4	4	4	4	4	2	30	Survived
	4	4	2	4	5	3	4	5	2	29	Survived
	5	5	X	4	2	1	5	2	4	23	Eliminated
	6	5	X	4	2	1	2	1	4	19	Eliminated
18	1	5	X	4	3	1	3	1	4	21	Eliminated
	2	4	3	4	4	2	4	3	2	26	Survived
	3	4	3	4	5	4	4	4	2	30	Survived
	4	4	2	4	5	3	4	5	2	29	Survived
	5	5	X	4	3	2	5	2	4	25	Eliminated
	6	5	X	4	2	1	3	1	4	20	Eliminated

5= excellent location, 4=minor concerns, 3=many minor/few major concerns, 2=several major concerns,
1= poor location, X= not at all viable, N/A= not applicable (location doesn't exist at this site)

TABLE 4: LOCATION SCREENING RESULTS (CONTINUED)

LOCK SITE	LOCK LOCA-TIONS	Construc-tion Ratings	Environ-mental Ratings	Geotech-nical Ratings	Hydrau-lics Ratings	Opera-tions Ratings	Real Estate Ratings	Civil Ratings	Struct-ural Ratings	Com-bined Ratings	Disposition
	1	5	5	5	1	1	1	1	1	20	Eliminated
	2	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
19	3	4	3	5	4	3	4	4	3	30	Survived
	4	4	1	-	1	1	4	1	3	15	Eliminated
	5	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	6	X	X	-	1	1	1	1	4	8	Eliminated
	1	5	3	5	3	2	3	1	4	26	Eliminated
	2	4	5	5	5	2	4	3	3	31	Survived
20	3	4	3	5	5	4	4	4	2	31	Survived
	4	4	3	4	4	3	4	4	3	29	Survived
	5	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	6	5	1	4	1	2	2	1	4	20	Eliminated
	1	5	4	4	2	1	1	1	4	22	Eliminated
	2	4	5	4	2	2	4	3	2	26	Survived
21	3	4	3	4	5	4	4	4	2	30	Survived
	4	4	3	4	4	3	4	5	2	29	Survived
	5	5	X	4	1	2	5	2	4	23	Eliminated
	6	5	X	4	1	1	4	1	4	20	Eliminated
	1	5	X	5	1	1	1	1	4	18	Eliminated
	2	4	5	5	3	2	4	3	3	29	Survived
22	3	4	3	5	4	4	4	4	3	31	Survived
	4	4	3	5	5	3	4	5	3	32	Survived
	5	5	X	4	1	2	5	2	4	23	Eliminated
	6	5	X	4	1	1	4	1	4	20	Eliminated
	1	4	3	1	2	3	1	1	5	20	Eliminated
	2	1	5	1	4	1	5	3	1	21	Survived
24	3	2	5	3	4	4	5	5	1	29	Survived
	4	5	5	4	5	5	5	5	2	36	Survived
	5	2	1	5	1	1	5	2	4	21	Eliminated
	6	1	1	1	1	1	1	5	3	14	Eliminated
	1	4	3	5	2	1	4	1	5	25	Survived
	2	1	5	1	4	2	5	3	1	22	Survived
25	3	4	5	2	4	4	5	5	1	30	Survived
	4	5	5	3	5	5	5	4	2	34	Survived
	5	2	1	4	1	1	5	2	4	20	Eliminated
	6	1	1	1	1	1	1	1	3	10	Eliminated
P	1	3	2	4	5	2	1	2	1	20	Survived
e	2	4	3	4	4	2	4	3	2	26	Survived
o	3	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
r	4	4	3	-	1	2	4	2	2	18	Eliminated
i	5	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
a	6	3	1	-	1	2	1	1	1	10	Eliminated
La	1	5	5	4	5	2	4	5	5	35	Survived
Gr	2	4	5	4	4	2	4	3	2	28	Survived
a	3	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
n	4	4	3	-	1	2	4	2	2	18	Eliminated
g	5	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
e	6	3	X	-	1	1	4	1	5	15	Eliminated

5= excellent location, 4= minor concerns, 3= many minor/few major concerns, 2= several major concerns,
1= poor location, X= not at all viable, N/A = not applicable (location doesn't exist at this site), *= later elimination

TABLE 5: REMAINING LOCATIONS FOR FURTHER STUDY						
Lock and Dam Site	Location Number					
	1	2	3	4	5	6
L/D 11	-	X	X	-	-	-
L/D 12	-	X	X	X	-	-
L/D 13	-	X	X	X	-	-
L/D 14[1]	-	X	-	X	-	-
L/D 15[1]	-	X	X	-	-	-
L/D 16	-	X	X	X	-	-
L/D 17	X	X	X	X	-	-
L/D 18	-	X	X	X	-	-
L/D 19[1]	-	-	X	-	-	-
L/D 20[2]	-	X	X	X	-	-
L/D 21	-	X	X	X	-	-
L/D 22[1]	-	X	X	X	-	-
L/D 24[2]	-	X	X	X	-	-
L/D 25	X	X	X	X	-	-
Peoria	X	X	-	-	-	-
La Grange	X	X	-	-	-	-

[1]These sites have rock foundations. All others (except for note 2) are sand-founded sites (requiring piles).
[2]These sites have mixed foundations; some locations would be rock-founded and some pile-founded.

LOCK SIZE SCREENING

The purpose of the size screening analysis was to assess the various lock size options and provide further screening of the potential measures in order to focus the study resources. This assessment looked at the 600-foot and 1,200-foot by 110-foot lock sizes for which conceptual designs and preliminary cost estimates already had been developed. Additionally, it examined other potential chamber sizes, either to verify that it was sufficient to move forward with the two sizes already identified or to identify other sizes for which conceptual designs and costs needed to be prepared.

The process verified that the only lock sizes meriting further examination were 110 feet by 1,200 feet and 110 feet by 600 feet. Longer, shorter, narrower, and wider locks were all examined and found to be subordinate to locks 110 feet wide by 1,200 feet long. In addition, if the need for a lower cost, “budget constrained” option was identified, the 110-foot by 600-foot locks were identified as the lowest cost size that still provides significant benefits. As a result, these two sizes were carried forward for further quantification of costs, benefits, and impacts.

Potential Chamber Sizes

The screening process first focused on identifying the potential chamber sizes to evaluate. The size and mix of tows presently using the lock system was used as an indicator of the chamber size needs. Many factors, both physical and economic, influence tow sizes. A summary of existing tow size data was developed and is shown in Tables 6 and 7.

TABLE 6: SELECTED TOW WIDTH DISTRIBUTIONS BY LOCK SITE			
Lock	Tow Width in Feet (Percentage)		
	< 40	40-75	75-110
Lock 11	3.56	11.40	85.04
Lock 15	15.32	12.22	72.46
Chamber 1	6.08	10.02	83.90
Chamber 4	70.74	25.39	3.88
Lock 19	2.44	11.12	86.44
Lock 20	3.59	10.38	86.04
Lock 21	5.60	12.18	82.22
Lock 22	2.68	13.20	84.12
Lock 24	2.21	15.30	82.49
Lock 25	2.49	15.41	82.10
Lock 26	7.39	18.16	74.45
La Grange	2.95	22.00	75.05

Source: Corps of Engineers Lock Performance Monitoring System (LPMS) Data.

TABLE 7: SELECTED TOW LENGTH DISTRIBUTIONS BY LOCK SITE							
Lock	Tow Length in Feet (Percentage)						
	<200	200-400	400-600	600-800	800-1000	1000-1,200	>1,200
Lock 11	1.58	3.92	13.69	14.61	19.54	46.62	0.05
Lock 15	1.16	19.87	11.53	11.47	15.07	40.90	0.00
Chamber 1	0.48	7.82	13.09	13.29	17.59	47.72	0.00
Chamber 4	5.23	92.05	2.13	0.58	0.00	0.00	0.00
Lock 19	0.60	4.50	7.75	7.90	12.46	66.80	0.00
Lock 20	1.15	5.17	7.20	7.61	9.73	69.14	0.00
Lock 21	0.75	7.82	6.71	8.24	9.85	66.60	0.03
Lock 22	0.70	4.72	8.29	8.79	9.55	67.93	0.03
Lock 24	0.27	4.66	8.94	10.46	9.61	66.06	0.00
Lock 25	0.48	4.63	8.98	10.78	9.51	65.62	0.00
Lock 26	3.77	7.46	9.48	13.64	12.80	52.81	0.02
La Grange	0.51	2.87	13.74	17.20	20.49	45.08	0.11

Source: Corps of Engineers Lock Performance Monitoring System (LPMS) Data.

The above data indicate that the most common tow length is nominally 1,200 feet and the most common width is nominally 110 feet. Tows can be locked through shorter chambers in multiple "cuts," but locking 110-foot-wide tows in narrower chambers is impractical due to coupling and operations requirements. Therefore, the minimum chamber width considered was 110 feet.

The jumbo hopper barge, which has a nominal length of 200 feet, is the predominant barge type on the UMR and IWW. Therefore, it was reasonable to consider chamber lengths in 200-foot increments. As a result, the cost and performance differences were evaluated between 200-, 400-, 600-, 800-, 1,000-, and 1,200-foot locks, all with 110-foot width. Vessels other than commercial tows also utilize system locks. These vessels include pleasure boats and passenger vessels. As a result, smaller non-commercial chambers devoted solely to these vessels also were evaluated.

It is also reasonable to question whether locks longer than 1,200 feet or wider than 110 feet might be economically advantageous. To answer this question required some systemic analysis of the benefits and costs. The benefits of operating increasingly larger tows are fairly limited. For

example, the barge-costing model by Reebie Associates, transportation management consultants, indicated that even doubling tow sizes to 30 barges as opposed to 15 would only provide a 30 percent cost reduction. If this practice were universally adopted from below Lock 19 to below St. Louis, the net present value of the savings obtained would be about $130 million (based on rough estimates of cost for tow trips through this reach times a 30 percent savings). However, much of the existing towboat fleet does not have the horsepower to adopt this practice, even if the channel were sufficient to accommodate this size tow, so the actual benefits are probably far less. In any event, this benefit is insufficient to provide for the added costs of modifying the two farthest downstream locks (Locks 26 and 27). Both Locks 26 and 27 have a 110-foot by 1,200-foot chamber and a 110-foot by 600-foot chamber in place, and these locks are not anticipated to need additional capacity in the near term. Given the system nature of movements, if larger locks were constructed upstream, these two locks also would need to be enlarged for the system to work efficiently. Therefore, it can be concluded that the 110-foot by 1,200-foot lock is the largest lock size that needs to be considered.

Costs Evaluated

The costs for 600-foot and 1,200-foot locks were prepared as part of the *Conceptual Lock Design* report and presented previously in this report. Cost estimates for other chamber sizes were developed as part of this analysis using interpolation/extrapolation of the 600- and 1,200-foot costs. Costs for all sizes evaluated are included as Attachment 1.

Another cost that was considered in the analysis was the impact to navigation during construction of a new chamber (i.e., delays to tows during periods that a lock is unavailable due to construction). These initial (prior to 2000) analyses indicated that impacts to navigation could be severe, up to $50 million or more for some lock locations and types. Impacts to navigation can influence the preferred lock locations and types. However, the impacts to navigation costs were not a deciding factor in the lock size screening.

Quantitative Size Screening Analysis

The methodology employed to compare choices of lock sizes was straightforward. The analysis used a model to evaluate the relative costs and benefits of various lock sizes, but it did not attempt to assess the justification of large-scale measures. (Such an analysis will be done using the system economic model.) Rather, the analysis estimated the incremental benefits of one large-scale measure compared to another. The basic assumption for the analysis was that all major systemic constraints would be relieved (measures implemented) in a timely fashion.

The analysis was conducted using the estimated construction costs and delay costs to estimate the reasonable time to implement and the useful life of the various large-scale improvement measures. From the point of implementation, the difference in benefits (delay cost reductions) between various potential measures was estimated over a 50-year project life. The costs of additional capital investment were also included as appropriate. For this analysis, the project life equals the time it takes under the assumed traffic growth and level of improvement for delay times to reach the pre-project level. As an example, if a particular improvement reduces delay from 10 hours per tow to 5 hours, but in 20 years traffic has grown such that delay is again 10 hours, the project life is shown as 20 years. Improvements with delay reductions lasting 50 years or more are shown with 50-year project lives.

The analysis was conducted using an economic model. The model was written and executed in three parts, all using TK Solver software. Part 1 estimated the traffic level at which implementation of a large-scale improvement was reasonable (i.e., what traffic level results in delay cost reductions exceeding construction costs). Part 2 prepared the stream of annual costs associated with transit times for the site, including an extension or an additional chamber for a 50-year period or until the limit of traffic was reached. Finally, Part 3 compared the cost streams for various alternatives and factored in the cost of future investments if needed.

Key Parameters/Assumptions

A. Tow Delay Cost $300 per hour for 4,000-4,400 hp tows (based on FY 1997 Planning Guidance Shallow Draft Vessel Cost).

B. Traffic Growth Rate 1.5 percent per annum (based on initial traffic forecast study results).

C. Discount Rate 7.375 percent (Federal rate in effect for FY 1997).

D. Lock processing times (estimated from LPMS data with appropriate modifications).

Results

To provide a representative sample, the model was run using information on Lock 22, Lock 25, Lock 19, and La Grange Lock. At Lock 22, Locations 2, 3, and 4 were evaluated. At Lock 25, Locations 1, 2, and 3 were evaluated. At La Grange Lock, Locations 1 and 2 were evaluated. Only Location 3 was evaluated at Lock 19. Completing the analysis at four sites provided some indication of the robustness of the basic conclusion that 1,200-foot by 110-foot locks are preferred. By evaluating Locks 22 and 25, the study team considered both a rock-founded and a pile-founded site. In addition, considering La Grange Lock allowed the study team to evaluate the impact of open pass, while considering Lock 19 accounted for a site where a 1,200-foot by 110-foot chamber already exists. All of these analyses support the conclusions.

The results of these runs are displayed in matrices (see Attachments 2 through 10). They provide the benefits gained (positive numbers) or foregone (negative numbers) of one chamber size over another. In addition, they show the estimated project life of the measure, the period of time the delay reductions remain below the pre-project level. Further, at Locks 22, 25, and La Grange, they show the comparison of 1,200-foot chambers at different locations; however, these only provide estimates of the relative values that will ultimately be determined through analysis with the system economic model. The project life is also displayed in these matrices.

For example, in looking at Attachment 3 for a lock extension at Lock 25, it is apparent that both the extensions to 800 feet and 1,000 feet provide very little benefits and have a very short project life of 3 years. In contrast, there are huge incremental benefits of over $200 million (present worth) in moving to a 1,200-foot chamber, which eliminated the need for time-consuming double lockages. This alternative in contrast has a project life of approximately 40 years before traffic grows such that delays again reach pre-project levels.

The matrices for the options showing new lock construction (Locations 1, 3, and 4) can be understood in a similar manner by again looking at changes in incremental benefits and project life. Attachment 6 evaluating options for Location 3 at Lock 22 shows that incremental benefits and project life increase as the size is increased. After the initial gain of benefits in getting to 400 feet,

the greatest incremental benefits accrue in moving to 600-foot chambers ($20 million) and 1,200-foot chambers ($39 million). The 800-foot and 1,000-foot lock options provided less relative incremental benefits, while the smaller 200- and 400-foot chambers had much shorter economic lives. Even for La Grange Lock, which due to site conditions has open pass conditions and a somewhat higher percentage of smaller tows, the incremental benefits in moving from a 1,000-foot to a 1,200-foot chamber of $7 million exceed the incremental costs of $5 million (for a Location 1 Type C lock).

Sensitivity Analysis

A sensitivity analysis was conducted to further verify that the lock size screening conclusions would hold over a range of varying parameters. Two very important parameters that were not varied in the initial evaluations were the growth rate (1.5 percent) and the discount rate (7.375 percent). The effects of varying these parameters (using Lock 22, Location 4 as an example) were tested (see Attachment 11). The attachment illustrates that the conclusions hold under a range of traffic growth rates (0 percent to 3.0 percent annually) and discount rates (5 percent to 9 percent). This sensitivity analysis provides additional confidence in the validity of the overall results of the analysis.

Conclusions of Size Screening

The results of this analysis in combination with information on system traffic, engineering options, and the professional judgement of study team allowed the following conclusions to be drawn regarding lock sizing:

1. In every case, for the preferred location and construction type, the 1,200-foot chamber provides enough incremental benefits to justify the incremental costs. The 1,200-foot locks are the preferred large-scale alternative in terms of net benefits.
2. While providing fewer benefits than 1,200-foot locks, 600-foot options could offer a “budget constrained” alternative. These locks represented the lowest first cost option at the time of this analysis that still provides significant net benefits. (However, the development of the revised lock extension (Type 2R), presented later in this report, provided a lower cost 1,200-foot option allowing 600-foot locks to be screened out during the secondary screening.)
3. When considering lock extensions, only extensions to 1,200 feet provide substantial benefits.
4. Locks larger than 110 feet by 1,200 feet will not provide enough incremental benefits to justify the additional costs.
5. Non-commercial (recreation) chambers represent a poor option relative to other chamber sizes.

As a result of the analysis and conclusions, the only lock sizes carried forward for further analysis were the 110-foot by 1,200-foot and 110-foot by 600-foot. This conclusion was reached based on the economic analysis summarized here as well as considering available information on traffic, engineering options, and the professional judgment of the study team.

LOCK TYPE SCREENING

The initial screening of lock types involved an iterative process. Two separate screening efforts and even the development of a new lock type occurred as efforts progressed and new information and insights were gained.

Initial Lock Screening of Types A, B, and C

The screening of lock types A, B, and C was conducted to eliminate any choice that was clearly dominated by another lock type at the same location.

At the start of this evaluation, it was noted that Type A locks are not a practical option for either Location 2 or Location 3. For Location 3, constructing a traditional cellular cofferdam around the entire construction area (per Type A design criteria) would severely encroach on the approach to the existing lock. Since such a cofferdam would be required to remain in place for an extended period of time, the adverse economic impact to commercial navigation during the construction period would be prohibitive. Placing a cofferdam around the existing lock to extend it to 1,200 feet would have worse economic consequences because there would be no access for navigation throughout the construction period. Therefore, Type A locks were eliminated at Locations 2 and 3.

However, the total number of measures needed to be further reduced to allow the more promising measures to be given more detailed attention. For that purpose, a screening of lock types was made. Whereas lock locations were screened based primarily upon factors over and above the basic lock construction (e.g., environmental concerns, channel realignment, relocations, etc.), lock types were screened primarily by comparing first costs for the basic lock construction at a given location. Cost estimates of the three lock types at five locations at a typical rock-founded site and a typical pile-founded site were presented in Tables 1 and 2. As can be noted from these tables, a large separation in first costs exists between locks of certain locations and certain types. These differences in first cost were found to be a sufficient basis to screen lock types. That is true because costs other than the basic lock cost (e.g., costs for channel work and environmental mitigation) are largely constant for all lock types at a given location, and lock performance does not vary greatly between lock types.

The lock type initial screening was completed using the costs discussed earlier. The following conclusions, applicable to both 600-foot and 1,200-foot locks, were drawn:

(1) Rock-Founded Lock Types. The separation in first cost between the lock Types B and C at Locations 2 and 3 was too small to eliminate either lock type at this point. For Location 4, however, the Type A lock is sufficiently more expensive than the Type B lock (without sufficient performance improvement) so that the Type A lock is eliminated from further consideration. For the remaining lock types, a more detailed analysis is required.

(2) Pile-Founded Lock Types. Based on the cost differentials, the following lock types are screened from further consideration:

Location 1 - Types A and B
Location 4 - Type A

After the above lock type screening, the remaining lock locations and types are as indicated below in Table 8.

TABLE 8: LOCATIONS AND TYPES REMAINING AFTER INITIAL LOCK TYPE SCREENING						
Lock and Dam Site	Location Number and Viable Types					
	1	2[1]	3	4	5	6
L/D 11	-	B, C	B, C	-	-	-
L/D 12	-	B, C	B, C	B, C	-	-
L/D 13	-	B, C	B, C	B, C	-	-
L/D 14[2]	-	B, C	-	B, C	-	-
L/D 15[2]	-	B, C	B, C	-	-	-
L/D 16	-	B, C	B, C	B, C	-	-
L/D 17	C	B, C	B, C	B, C	-	-
L/D 18	-	B, C	B, C	B, C	-	-
L/D 19[2]	-	-	B, C	-	-	-
L/D 20[3]	-	B, C	B, C	B, C	-	-
L/D 21	-	B, C	B, C	B, C	-	-
L/D 22[2]	-	B, C	B, C	B, C	-	-
L/D 24[3]	-	B, C	B, C	B, C	-	-
L/D 25	C	B, C	B, C	B, C	-	-
Peoria	C	B, C	-	-	-	-
La Grange	C	B, C	-	-	-	-

[1]There are no new 600-foot lock alternatives at Location 2. Therefore, the Location 2 column is blank for the 600-foot lock case (the 1,200-foot lock case is shown). All other columns are the same for both lock sizes.
[2]These sites have rock foundations. All others (except for note 2) are sand-founded sites (requiring piles).
[3]These sites have mixed foundations; some locations would be rock-founded and some pile-founded.

Continuation of Lock Type Screening

Additional analysis and quantification of benefits and costs allowed some additional screening. During the expert elicitation process (February 1997) used to determine the delays to navigation during construction, further justification for screening lock alternatives was identified. As a result, the pile-founded Location 2, Type C and Location 3, Type B locks were eliminated due to the reasons discussed below. As a result, more detailed cost information was not developed for these choices. (Subsequent analysis of the impacts to navigation revealed that these costs might be less of a factor than initially anticipated. As a result, the screening discussed below, which was used to eliminate Location 2, Type C locks and Location 3, Type B locks at pile-founded sites, is weakened. As shown later in this report, the revised Type 2R lock would have screened out the 2C lock option regardless. However, for Location 3 locks, if subsequent analysis shows justification for locks at this location, both Types B and C will be considered during the detailed site-specific design efforts.) Additional engineering efforts at Lock 19 also revealed the ability to eliminate 600-foot locks and Location 3, Type B 1,200-foot locks at this site.

The Location 2, Type C lock features a weeping floor slab system composed of numerous precast blocks, filter layers, and evenly spaced pile-founded struts between the lockwalls that resist tension and compression forces. The Type B lock features large precast box-beam-like slabs that span

from wall to wall. The navigation impacts to construct the two floor systems were subjectively determined by experts. Compared to the Type B floor system, the Type C system would require a much longer time to install, would require additional winter-time lock closure with dewatering, and overall would be more complicated to construct. Although the Type C lock is slightly cheaper to construct than the Type B lock, the economic impacts to navigation during construction and increased risks and uncertainty outweigh the savings in construction cost. Thus, all of the pile-founded Location 2, Type C locks were eliminated from further consideration.

For Location 3, both lock Types B and C feature an extension of the existing downstream guidewall. However, the Type B lock features a permanent extension of the existing downstream guidewall, and the Type C a temporary one. The permanent construction offers a fixed location with high first cost. Navigation industry representatives reviewing this plan concluded that, after the lock at Location 3 is complete, the wall would likely hinder upbound approaches to the lock. Thus, the wall would have to be removed, resulting in additional costs. The permanent wall also provides for no adjustment in its location to suit navigation in the interim. The temporary wall consists of a series of connected barges that are moored with spud piles. The Type C "spud barge wall" is advantageous in that it is easily installed and removed, cheaper than permanent construction, and adjustable to suit navigation needs. (This type of guidewall extension was successfully used at the old Lock 26 on the Mississippi River on a temporary basis.) The Type C lock also features a little less concrete in the lockwall than a Type B lock, which would reduce the walk/storage surface available on the walls. Otherwise, the Type B and Type C locks are the same. The guidewall associated with the Type C lock is preferred mainly due to its lower cost and the advantages of the temporary guidewall extension versus a permanent wall. Basically, the Type C offers preferred features at a lower cost than does its competitor; therefore, it is a preferred choice. All of the pile-founded Location 3, Type B locks are eliminated from further consideration.

In addition, the following was determined regarding Lock 19, a rock-founded site. Due to the higher lift at Lock 19 (38 feet maximum), the Type C, Location 3 lock concept (consisting of sheet pile cellular lockwalls) is not practical and was eliminated from consideration. While an alternative concept could be developed (or the Type B lock economized), this refining effort is considered unwarranted for the system study. In addition, due to the high relative cost to construct and the lower performance of 600-foot locks at this high lift site, constructing a new 600-foot lock was eliminated as an option at Lock 19.

The array of lock choices remaining after the screening steps noted above is shown in Table 9.

TABLE 9: LOCK LOCATIONS AND TYPES REMAINING AFTER CONTINUED LOCK TYPE SCREENING

Lock and Dam Site	Location Number and Viable Types					
	1	2[1]	3	4	5	6
L/D 11	-	B	C	-	-	-
L/D 12	-	B	C	B, C	-	-
L/D 13	-	B	C	B, C	-	-
L/D 14[2]	-	B, C	-	B, C	-	-
L/D 15[2]	-	B, C	B, C	-	-	-
L/D 16	-	B	C	B, C	-	-
L/D 17	C	B	C	B, C	-	-
L/D 18	-	B	C	B, C	-	-
L/D 19[2]	-	-	B	-	-	-
L/D 20[3]	-	B, C	B, C	B, C	-	-
L/D 21	-	B	C	B, C	-	-

L/D 22[2]	-	B, C	B, C	B, C	-	-
L/D 24[3]	-	B, C	B, C	B, C	-	-
L/D 25	C	B	C	B, C	-	-
Peoria	C	B	-	-	-	-
La Grange	C	B	-	-	-	-

[1]There are no new 600-foot lock alternatives at Location 2. Therefore, this column is blank for the 600-foot lock case (the 1,200-foot lock case is shown). All other columns are the same for both lock sizes.
[2]These sites have rock foundations. All others (except for note 3) are sand-founded sites (requiring piles).
[3]These sites have mixed foundations; some locations would be rock-founded and some pile-founded.

Additional Design Efforts - Lock Extension Type R

Through the evaluation of the lock concepts discussed above, it became apparent that the lowest first cost lock choice was a lock extension; however, the type of construction required delays/closures to the navigation that were found to be excessively high. The results of these investigations highlighted the need for continued engineering and design with a focus on significantly reducing the cost of impacts to navigation during the construction of a lock extension.

The study team further investigated lock extension designs to identify opportunities to minimize the delays. Additional departures from standard criteria were considered to lower costs and reduce impacts to navigation. This resulted in a Location 2, Type R lock with the following features:

1. The existing F&E culverts are not extended into the new lockwalls.
2. The existing intermediate wall is extended with concrete-filled sheet pile cells or steel cylindrical shells.
3. The existing downstream guidewall is modified and strengthened for reuse as a lockwall.
4. The new lower guidewall is only 600 feet long and consists of concrete beams spanning cells.
5. The new lower gate monoliths are float-in units that would be installed in the wet.
6. Approach channel improvements would not be made unless necessary to utilize the lock (in which case they would be channel work, not improvements, due to their necessity).

Key decisions for these revised concepts were to maintain the elevation of the existing lock floor and leave the sill depth unchanged (1.38 times draft for clearance over the sill at minimum tail water). These decisions allowed the existing lower guidewall to be incorporated into the design as part of the landside lockwall, since lowering the sill and chamber depth would have undermined the wall and necessitated its removal. As a result of this additional design effort, Type R locks were added as an option at Location 2. See Table 10 below.

TABLE 10: LOCK LOCATIONS AND TYPES CARRIED FORWARD FOR FURTHER QUALITATIVE EVALUATION

Lock and Dam Site	Location Number and Viable Types					
	1	2[1]	3	4	5	6
L/D 11	-	B, R	C	-	-	-
L/D 12	-	B, R	C	B, C	-	-
L/D 13	-	B, R	C	B, C	-	-
L/D 14[2]	-	B, C, R	-	B, C	-	-
L/D 15[2]	-	B, C, R	B, C	-	-	-
L/D 16	-	B, R	C	B, C	-	-
L/D 17	C	B, R	C	B, C	-	-
L/D 18	-	B, R	C	B, C	-	-
L/D 19[2]	-	-	B, C	-	-	-
L/D 20[3]	-	B, C, R	B, C	B, C	-	-

L/D 21	-	B, R	C	B, C	-	-
L/D 22[2]	-	B, C, R	B, C	B, C	-	-
L/D 24[3]	-	B, C, R	B, C	B, C	-	-
L/D 25	C	B, R	C	B, C	-	-
Peoria	C	B, R	-	-	-	-
La Grange	C	B, R	-	-	-	-

[1]There are no new 600-foot lock alternatives at Location 2. Therefore, this column is blank for the 600-foot lock case (the 1,200-foot lock case is shown). All other columns are the same for both lock sizes.
[2]These sites have rock foundations. All others (except for note 3) are sand-founded sites (requiring piles).
[3]These sites have mixed foundations; some locations would be rock-founded and some pile-founded.

For each of the remaining lock design choices, costs and performance factors were further quantified and are shown following the wicket dam discussion.

OTHER LARGE- SCALE OPTIONS SCREENING

Wicket Dam Measures

In addition to lock extensions and new locks, Locks and Dams 17 and 20 were considered for a navigable dam based upon historic flow records. For the period of time that flows are sufficient for open pass, a navigable dam allows tows to save time by bypassing the lock. One limitation to open pass is that it is not schedulable very far in advance. On the Mississippi, there can be about 2 weeks of notice based upon river forecasting models. In any given year, there may be no open pass conditions. Other years, there may be extended periods of open pass. Another limitation concerns the wintertime closure experienced each year. That is, if the flow conditions for open pass occur during the wintertime closure period, there would be minimal to no traffic on the river to benefit from these conditions. Also, during very high flows and open pass at Locks and Dams 17 and 20, the flows would be too high to allow navigation at other locks or channel reaches. This rare case occur because of adverse currents or because the lockwalls at the other locks become inundated. Thus, there would be no system benefits to the open pass at these times.

Table 11 indicates the average annual occurrence of open pass conditions at Locks and Dams 17 and 20. Since there is a significant cost in mobilizing to lower the wickets and remobilizing to raise them again, this operation would not be done if the open pass conditions were expected to last only a few days, which is likely to be the case in most months.[1]

TABLE 11: COST AND PERFORMANCE OF WICKET DAM MEASURES

Lock	Percent Open Pass[1]	Equivalent Days per Month[1]	First Cost Constr. and Equipment[2] ($1,000)	Operations & Maint. Costs[3] ($1,000)	Site-Specific Env. Cost ($1,000)[4]	Total Cost ($1,000)
Lock 17 Average (Mar-Dec)	23.5	7.2	$62,400	$4,510	$8,250	$75,160
Lock 20 Average (Mar-Dec)	13.0	4.0	$86,100	$7,470	$4,125	$97,695

[1] Potential open pass conditions, based on average by month for period of record 1936 through 1996.

[1] The determination of the value of lowering the wickets to allow open pass navigation would depend upon the extent of the queue built up at the time. Therefore, a specific number of days of high flows above which lowering the dam would be worthwhile cannot be set at this time. In addition, the flow capacity of the wicket dam section may be needed to prevent a backwater effect that would raise the upper pool. Thus, some of the wickets may have to be lowered anyway.

[2] Costs are at 1996 price level. Costs shown only include the first cost of construction and purchase of equipment to operate the wickets (towboat, maneuver boat, and barge-mounted backhoe/hook).

[3] Present worth of operations and maintenance costs are shown based on discount rate of 7.4 percent. Annual operating cost of $61,600 would be incurred to raise and lower the wickets an average of five times each during the period of March through July each year. This annual cost includes labor for a crew of five people, fuel costs, and equipment charges. In addition, annual maintenance cost assumes need for replacement of 15 percent of the wicket gates/year (5 gates at Lock and Dam 17 at a cost of $550,000 and 9 gates at Lock and Dam 20 at a cost of $990,000). This maintenance cost would not be incurred until about the 10th year of operation. No major rehabilitation is expected to be required within the 50-year planning horizon.

[4] Environmental costs shown are habitat replacement values, based on the *Site-Specific Habitat Assessment* report for Lock 20 and extrapolation of earlier estimates for Lock 17.

Wicket Dam Screening

The wicket dam option was evaluated in terms of the benefits produced compared to the costs, also taking into consideration other relevant qualitative factors. Table 11 summarizes the performance and costs of the wickets, plus it includes the present worth of operations and maintenance and costs of site-specific environmental habitat replacement. The environmental costs for Lock 20 are habitat replacement values developed as part of the *Site-Specific Habitat Assessment* report. Site-specific costs for Lock 17 are shown as double the Lock 20 costs. This is a rough estimate based on the fact that a popular walleye fishing area at Lock 17 would be impacted downstream, and the State and Federal resource agencies expressed strong opposition to closing connecting channels in the area of Turkey Island and Turkey Chute.

The wicket dams were screened out based on limitations in performance for the relatively high cost and site-specific environmental resource concerns. The costs are over one-half of the costs of new Location 2R locks. Wicket dams only provide navigation benefits when river stages are such that the wickets can be lowered and allow open pass. As the table shows, the percent of the navigation season during which this is anticipated to occur is relatively limited as well as it being unpredictable, especially at Lock 20. Even during open pass, the effective system benefits are limited by the performance of the adjacent upstream and downstream locks. In addition, during shorter periods of potential open pass conditions (only a few days) the cost and time required to just lower and raise the gates would significantly diminish the benefits. While Lock 17 shows somewhat lower costs and greater performance, the greater environmental concerns at this site reinforce the decision to screen out this measure from further consideration.

SECTION 3 - FURTHER QUANTIFICATION OF PERFORMANCE AND COST

With lock construction choices narrowed to the several most promising at each site, it was no longer sufficient to compare them on the basis of first cost of construction alone. Instead, after the initial lock location, size, and type screening was completed, it became necessary to further quantify costs, impacts, and performance - "time savings benefits." In considering all of the factors that needed to be quantified to compare choices, the following Lock Design Evaluation Factors were developed.

TABLE 12: LOCK DESIGN EVALUATION FACTORS
Cost Factors
First Costs
Basic Lock & Guidewall Construction
Channel Work and Levees (as needed)
Relocation & Real Estate Requirements
Replacement Costs (as needed)
Maintenance and Repair Costs
Routine Maintenance
Major Rehabilitation
Operation Costs
Economic Impacts to Navigation During Construction (delays and closures)
Environmental Resource Impacts
Cultural and Historic Resource Impacts
Benefits Factors
Performance
Lockage Time
Disposition of Existing Lock

While the remainder of this section provides additional descriptions and detail, Table 13 summarizes the cost information and performance (lockage times) for each of the remaining lock locations and types. The 16 sites shown in the table were identified in the reconnaissance studies as having a potential need for new lock construction within the planning horizon.

Although at first look it might appear that all of the lock and dam sites are similar, each site is unique in at least some aspects. The limited field investigations of this system study did not allow complete consideration of all site-specific conditions. Some of the site design considerations were considered, but many of these items need to be looked at in greater detail during site-specific feasibility study if project justification is shown as part of the system study. Additional foundation exploration is especially important for the foundation design and accuracy of the cost estimates.

TABLE 13: SUMMARY OF PERFORMANCE AND COST									
				Costs ($1,000 in 1996 Price Level)					
Lock Site/ Length	Lock Location	Type	Avg Lockage Time[1]	First Cost (lock and guidewalls)[2]	First Cost Channel Work and Levees	First Cost Real Estate and Reloca-tions	Operation and Maint. (Present Worth)[3]	Major Rehab. (Present Worth)	Total Cost w/out Env or Impacts to Nav[4]
LaGrg.	Existing Lock		103						
1,200ft	Loc 1	1C	60	$145,000	$17,000	$500	$5,900	$1,000	$169,400
	Loc 2	2B	57	$152,000	$0	$50	$0	$0	$152,050
		2R	60	$110,000	$0	$50	$0	$4,200	$114,250
600 ft	Loc 1	1C	105	$131,000	$17,000	$500	$5,900	$700	$155,100
Peoria	Existing Lock		107						
1,200ft	Loc 1	1C	64	$144,000	$3,700	$18,900	$5,900	$1,000	$173,500
	Loc 2	2B	59	$146,000	$500	$3,500	$0	$0	$150,000
		2R	62	$117,000	$0	$50	$0	$4,200	$121,250
600 ft	Loc 1	1C	112	$131,000	$3,700	$18,900	$5,900	$700	$160,200
25	Existing Lock		101						
1,200ft	Loc 1	1C	56	$165,000	$9,300	$2,800	$5,900	$1,000	$184,000
	Loc 2	2B	51	$160,000	$1,000	$200	$0	$0	$161,200
		2R	54	$119,000	$1,000	$200	$0	$4,200	$124,400
	Loc 3	3C	50	$195,000	$1,000	$30	$5,900	$1,000	$202,930
	Loc 4	4B	50	$282,000	$0	$30	$5,900	$0	$287,930
		4C	50	$248,000	$0	$30	$5,900	$10,700	$264,630
600ft	Loc 1	1C	108	$151,000	$13,000	$2,800	$5,900	$700	$173,400
	Loc 3	3C	99	$160,000	$1,000	$30	$5,900	$700	$167,630
	Loc 4	4B	99	$231,000	$0	$30	$5,900	$0	$236,930
		4C	99	$217,000	$0	$30	$5,900	$7,000	$229,930
24	Existing Lock		107						
1,200ft	Loc 2	2B	53	$133,000	$800	$30	$0	$0	$133,830
		2C	53	$131,000	$800	$30	$0	$1,000	$132,830
		2R	56	$116,000	$0	$30	$0	$4,200	$120,230
	Loc 3	3B	54	$195,000	$800	$30	$5,900	$0	$201,730
		3C	54	$189,000	$800	$30	$5,900	$3,000	$198,730
	Loc 4	4B	51	$253,000	$800	$30	$5,900	$0	$259,730
		4C	51	$251,000	$800	$30	$5,900	$10,700	$268,430
600ft	Loc 3	3B	105	$162,000	$800	$30	$5,900	$0	$168,730
		3C	105	$150,000	$800	$30	$5,900	$2,000	$158,730
	Loc 4	4B	105	$225,000	$800	$30	$5,900	$0	$231,730
		4C	105	$233,000	$800	$30	$5,900	$7,000	$246,730
22	Existing Lock		112						
1,200ft	Loc 2	2B	63	$117,000	$4,800	$30	$0	$0	$121,830
		2C	63	$110,000	$4,800	$30	$0	$1,000	$115,830
		2R	68	$97,000	$0	$30	$0	$4,200	$101,230
	Loc 3	3B	64	$131,000	$5,100	$30	$5,900	$0	$142,030
		3C	64	$124,000	$5,100	$30	$5,900	$3,000	$138,030
	Loc 4	4B	63	$194,000	$6,900	$30	$5,900	$0	$206,830
		4C	63	$182,000	$6,900	$30	$5,900	$5,200	$200,030
600ft	Loc 3	3B	107	$109,000	$5,100	$30	$5,900	$0	$120,030
		3C	107	$96,000	$5,100	$30	$5,900	$2,000	$109,030
	Loc 4	4B	107	$182,000	$6,900	$30	$5,900	$0	$194,830
		4C	107	$173,000	$6,900	$30	$5,900	$3,500	$189,330
21	Existing Lock		98						
1,200ft	Loc 2	2B	51	$149,000	$2,000	$60	$0	$0	$151,060
		2R	55	$108,000	$0	$60	$0	$4,200	$112,260
	Loc 3	3C	57	$188,000	$2,200	$60	$5,900	$1,000	$197,160
	Loc 4	4B	54	$277,000	$5,500	$30	$5,900	$0	$288,430
		4C	54	$267,000	$5,500	$30	$5,900	$10,700	$289,130
600ft	Loc 3	3C	95	$154,000	$2,200	$60	$5,900	$700	$162,860
	Loc 4	4B	95	$227,000	$5,500	$30	$5,900	$0	$238,430
		4C	95	$235,000	$5,500	$30	$5,900	$7,000	$253,430

TABLE 13 (Continued)

Lock Site/ Length	Lock Location	Type	Avg Lockage Time[1]	Costs ($1,000 in 1996 Price Level) First Cost (lock and guidewalls)[2]	First Cost Channel Work and Levees	First Cost Real Estate and Reloca-tions	Operation and Maint. (Present Worth)[3]	Major Rehab. (Present Worth)	Total Cost w/out Env or Impacts to Nav[4]
20	Existing Lock		104						
1,200ft	Loc 2	2B	54	$103,000	$13,200	$3,700	$0	$0	$119,900
		2C	54	$102,000	$13,200	$3,700	$0	$1,000	$119,900
		2R	58	$90,000	$3,100	$1,900	$0	$4,200	$99,200
	Loc 3	3B	54	$132,000	$10,200	$3,000	$5,900	$0	$151,100
		3C	54	$134,000	$10,200	$3,000	$5,900	$3,000	$156,100
	Loc 4	4B	53	$189,000	$6,300	$30	$5,900	$0	$201,230
		4C	53	$175,000	$6,300	$30	$5,900	$10,700	$197,930
600ft	Loc 3	3B	101	$110,000	$10,200	$3,000	$5,900	$0	$129,100
		3C	101	$106,000	$10,200	$3,000	$5,900	$2,000	$127,100
	Loc 4	4B	101	$158,000	$6,300	$30	$5,900	$0	$170,230
		4C	101	$156,000	$6,300	$30	$5,900	$7,000	$175,230
19	Existing Lock		58						
1,200ft	Loc 3	3B	68	$265,000	$6,100	$50	$5,900	$0	$277,050
18	Existing Lock		99						
1,200ft	Loc 2	2B	52	$143,000	$300	$3,100	$0	$0	$146,400
		2R	55	$103,000	$100	$3,100	$0	$4,200	$110,400
	Loc 3	3C	52	$190,000	$300	$50	$5,900	$1,000	$197,250
	Loc 4	4B	50	$274,000	$100	$50	$5,900	$0	$280,050
		4C	50	$256,000	$100	$50	$5,900	$10,700	$272,750
600ft	Loc 3	3C	98	$156,000	$300	$50	$5,900	$700	$162,950
	Loc 4	4B	97	$225,000	$100	$50	$5,900	$0	$231,050
		4C	97	$225,000	$100	$50	$5,900	$7,000	$238,050
17	Existing Lock		105						
1,200ft	Loc 1	1C	57	$150,000	$20,400	$400	$5,900	$1,000	$177,700
	Loc 2	2B	55	$148,000	$8,900	$200	$0	$0	$157,100
		2R	59	$107,000	$6,200	$200	$0	$4,200	$117,600
	Loc 3	3C	53	$186,000	$11,900	$200	$5,900	$1,000	$205,000
	Loc 4	4B	54	$275,000	$11,900	$200	$5,900	$0	$293,000
		4C	54	$267,000	$11,900	$200	$5,900	$10,700	$295,700
600ft	Loc 1	1C	104	$137,000	$25,000	$400	$5,900	$700	$169,000
	Loc 3	3C	101	$153,000	$11,900	$200	$5,900	$700	$171,700
	Loc 4	4B	101	$226,000	$11,900	$200	$5,900	$0	$244,000
		4C	101	$235,000	$11,900	$200	$5,900	$7,000	$260,000
16	Existing Lock		100						
1,200ft	Loc 2	2B	51	$149,000	$13,600	$300	$0	$0	$162,900
		2R	55	$119,000	$1,000	$300	$0	$4,200	$124,500
	Loc 3	3C	50	$189,000	$15,500	$300	$5,900	$1,000	$211,700
	Loc 4	4B	50	$267,000	$16,000	$300	$5,900	$0	$289,200
		4C	50	$265,000	$16,000	$300	$5,900	$10,700	$297,900
600ft	Loc 3	3C	98	$155,000	$15,500	$300	$5,900	$700	$177,400
	Loc 4	4B	98	$219,000	$16,000	$300	$5,900	$0	$241,200
		4C	98	$233,000	$16,000	$300	$5,900	$7,000	$262,200
15	Existing Lock		111						
1,200ft	Loc 2	2B	59	$104,000	$1,400	$30	$0	$0	$105,430
		2C	59	$96,000	$1,400	$30	$0	$1,000	$98,430
		2R	62	$85,000	$1,400	$30	$0	$4,200	$90,630
	Loc 3	3B	62	$129,000	$1,400	$30	$5,900	$0	$136,330
		3C	62	$151,000	$1,400	$30	$5,900	$3,000	$161,330
600ft	Loc 3	3B	109	$106,000	$1,400	$30	$5,900	$0	$113,330
		3C	109	$119,000	$1,400	$30	$5,900	$2,000	$128,330

TABLE 13 (Continued)

Lock Site/ Length	Lock Location	Type	Avg Lockage Time[1]	Costs ($1,000 in 1996 Price Level) First Cost (lock and guidewalls)[2]	First Cost Channel Work and Levees	First Cost Real Estate and Reloca-tions	Operation and Maint. (Present Worth)[3]	Major Rehab. (Present Worth)	Total Cost w/out Env or Impacts to Nav[4]
14	Existing Lock		104						
1,200ft	Loc 2	2B	56	$104,000	$6,300	$30	$0	$0	$110,330
		2C	56	$99,000	$6,300	$30	$0	$1,000	$106,330
		2R	59	$92,000	$6,300	$30	$0	$4,200	$102,530
	Loc 4	4B	57	$192,000	$9,800	$500	$5,900	$0	$208,200
		4C	57	$182,000	$9,800	$500	$5,900	$5,200	$203,400
600ft	Loc 4	4B	103	$172,000	$9,800	$500	$5,900	$0	$188,200
		4C	103	$170,000	$9,800	$500	$5,900	$3,500	$189,700
13	Existing Lock		92						
1,200ft	Loc 2	2B	46	$146,000	$0	$400	$0	$0	$146,400
		2R	49	$106,000	$0	$400	$0	$4,200	$110,600
	Loc 3	3C	47	$187,000	$2,000	$400	$5,900	$1,000	$196,300
	Loc 4	4B	47	$273,000	$1,500	$400	$5,900	$0	$280,800
		4C	47	$252,000	$1,500	$400	$5,900	$10,700	$270,500
600ft	Loc 3	3C	93	$153,000	$2,000	$400	$5,900	$1,000	$162,300
	Loc 4	4B	93	$224,000	$1,500	$400	$5,900	$0	$231,800
		4C	93	$221,000	$1,500	$400	$5,900	$7,000	$235,800
12	Existing Lock		95						
1,200ft	Loc 2	2B	47	$145,000	$800	$300	$0	$0	$146,100
		2R	50	$116,000	$800	$300	$0	$4,200	$121,300
	Loc 3	3C	47	$187,000	$4,700	$300	$5,900	$1,000	$198,900
	Loc 4	4B	46	$268,000	$4,400	$300	$5,900	$0	$278,600
		4C	46	$254,000	$4,400	$300	$5,900	$10,700	$275,300
600ft	Loc 3	3C	93	$153,000	$4,700	$300	$5,900	$700	$164,600
	Loc 4	4B	93	$220,000	$4,400	$300	$5,900	$0	$230,600
		4C	93	$223,000	$4,400	$300	$5,900	$7,000	$240,600
11	Existing Lock		102						
1,200ft	Loc 2	2B	49	$104,000	$5,700	$400	$0	$0	$110,100
		2R	53	$75,000	$1,600	$400	$0	$4,200	$81,200
	Loc 3	3C	49	$190,000	$5,700	$400	$5,900	$1,000	$203,000
600ft	Loc 3	3C	100	$156,000	$5,700	$400	$5,900	$700	$168,700
Avg All Existing Lock			103						
Averages New 1,200-foot Locks									
1,200 ft	Loc 1	1C	59	$151,000	$12,600	$5,650	$5,900	$1,000	$176,150
	Loc 2	2B	54	$133,533	$3,953	$822	$0	$0	$138,309
		2C	57	$107,600	$5,300	$764	$0	$1,000	$114,664
		2R	57	$104,000	$1,433	$472	$0	$4,200	$110,105
	Loc 3	3B	59	$146,750	$4,375	$773	$5,900	$0	$157,798
		3C	53	$175,833	$5,067	$403	$5,900	$1,667	$188,869
	Loc 4	4B	52	$249,455	$5,745	$173	$5,900	$0	$261,273
		4C	52	$236,273	$5,745	$173	$5,900	$9,700	$257,791
Averages New 600-foot Locks									
600 ft	Loc 1	1C	107	$137,500	$14,675	$5,650	$5,900	$700	$164,425
	Loc 3	3B	106	$121,750	$4,375	$773	$5,900	$0	$132,798
		3C	100	$142,583	$5,067	$403	$5,900	$1,158	$155,111
	Loc 4	4B	99	$209,909	$5,745	$173	$5,900	$0	$221,727
		4C	99	$211,000	$5,745	$173	$5,900	$6,364	$229,182

Notes:
[1]Average lockage times shown are based on equal percentages of fly, exchange, and turnback lockages. The numbers shown assume turnback approaches occur during the lockage of the previous tow. This may slightly understate overall lockage time.
[2]The project element costs are based on 1996 prices and include 25 percent contingencies.
[3]The Operations and Maintenance Costs and the Major Rehab Costs reflect only the incremental life cycle costs for each alternative discounted at 7.4 percent. Total O&M costs would be $13,100 million for Location 2 locks and $19,000 million for other locations leading to the existence of two locks. However, this $13,100 million associated with Location 2 would be required for O&M of the existing lock and as such does not represent a with-project cost.
[4]The total costs shown are not all inclusive. They do not include the costs related to environmental impacts, cultural impacts, impacts to navigation during construction, nor do they include updated real estate costs that were under development at the time of publication.

The individual cost factors and performance improvements are reviewed below. Each of the cost factors, as applicable, was quantified in monetary terms on a present worth basis (using a 7.4% interest rate) for the 50-year planning horizon for both 600- and 1,200-foot locks. Following the discussion of costs, the performance improvements were quantified in terms of minute changes in lockage times and disposition of the existing lock.

COST FACTORS

First Costs. Includes those costs associated with the initial construction of an improvement.

Costs of Basic Lock and Guidewall Construction. First costs for the lock concepts were developed for and are presented in Appendix A of the *Conceptual Lock Designs* report and summarized in Table 13 above. Costs for the 600-foot locks were developed by applying a ratio relating costs of 1,200-foot chambers to costs for 600-foot chambers based on detailed evaluations of both sizes at Locks 22 and 25.

Costs for Channel Work and Levees. For most lock construction, some channel work upstream and downstream is required. This channel work can be extensive for a Location 1 lock, but usually is relatively minor for Locations 3 or 4 where only a slight shift in the channel may be needed. For some sites, the existing approach conditions are poor. Thus, an upgrade in the channel design would be recommended along with construction of a Location 2 lock. The channel requirements (and costs) are the same for 600-foot locks as for 1,200-foot locks since the tows have to travel the same path regardless of the lock size. Levee relocation costs are only necessary at some Location 1 locks.

Costs for Relocation and Real Estate Requirements. The right-of-way and relocations cost is another part of the first cost that was a relatively small component for most total lock costs. Estimates of the value of the lands that would need to be purchased were made based on estimates of the acreage at each lock and dam site required for construction. These estimates are preliminary and would need to be revisited if any large-scale measures are recommended for implementation.

Replacement Costs. The possibility of lock replacement in the 50-year planning period was investigated. Of particular concern were the locks that would be constructed with steel sheet pile cells, and the possibility of corrosion threatening the integrity of the cells. A study was conducted at the Corps of Engineers Construction Engineering Research Laboratory (CERL) to address this concern (Reference 26). It found that if no special design precautions are taken, the life of sheet pile cells for locks would be about 30 years after which they can be rehabilitated to extend their useful life. None of the lock concepts was found to require a complete replacement within the 50-year planning period; therefore, this factor dropped out of consideration.

Maintenance and Repair Costs. These costs are discussed in their two basic categories below.

Routine Maintenance. Routine or normal maintenance occurs frequently and includes grounds keeping, building maintenance, and equipment maintenance. These items would not vary significantly from one type of lock to another, and the maintenance estimates were uniform, except as noted below. The additional wall monoliths of a 1,200-foot lock are low maintenance sections, so the maintenance costs of 600-foot and 1,200-foot locks are essentially equal.[2] Therefore, routine

[2] While a longer chamber represents more total structure, this is offset by the fact that a 600-foot lock will go through many more lock operations than a 1,200-foot lock to pass the same traffic. This added "wear and tear" affects the lock components that require the most maintenance (machinery, gates, valves, etc.).

maintenance is not a significant screening factor. The only significant maintenance cost difference is between maintenance of a lock at Location 2 (a single 1,200-foot lock) and maintenance of a 1,200-foot lock at Locations 1, 3, or 4 plus maintenance of the existing lock (i.e., maintaining two locks). The annual maintenance cost for a Location 2 (single 1,200-foot lock) is estimated to average $197,000.[3] The annual maintenance cost for the existing 600-foot lock plus a 1,200-foot lock at either Locations 1, 3, or 4 is estimated to average $394,000 ($197,000 each for the two locks).[4] However, the $197,000 annual cost of maintaining one lock (for Location 2 options) is a without-project cost and as such does not represent a new with-project cost since it would be incurred regardless. The only cost to be accounted for in the with-project condition is the additional $197,000 annually associated with options resulting in an additional lock (Locations 1, 3, or 4).

Major Rehabilitation. If only routine maintenance is performed, the lock structures would deteriorate to the point where they are less reliable and less able to efficiently perform their function of locking boats. As noted earlier, most of the existing locks were built in the 1930's and are still functioning. However, these locks had deteriorated significantly since their construction and needed to have maintenance above the routine level at about 50 years of age. For simplicity, this discussion will refer to any of this type of extra maintenance as major rehabilitation.

Since major rehabilitation costs are incurred in the future, they are discounted to present values. For example, a $50 million major rehabilitation contract in year 50 would be equivalent to $1.443 million today. While this is a large cost, it is relatively insignificant compared to the first cost of construction that is typically at least $100 million. Nevertheless, the major rehabilitation costs were estimated for those lock concepts that are expected to require major rehabilitation within the planning period.

The following are general descriptions of the major rehabilitation for the lock types that are expected to require this above normal maintenance. The associated costs for this work are shown in Table 14 for 1,200-foot and 600-foot locks. These items include rehabilitating sheet pile cell walls, replacing concrete lockwall panels, and grouting. The actual future rehabilitation needs for a lock are very uncertain and depend upon a number of site-specific factors such as: quality of original construction (influenced by quality of materials, construction methods and workmanship, weather during construction, etc.), environmental factors (freeze-thaw cycles, corrosive influences, predominant weather), random events (accidents, extreme weather), and extent and timing of routine maintenance. The values in Table 14 are for the purposes of screening, but updated estimates will likely be required and documented in the Engineering Appendix.

[3] The present worth of 50 years of routine maintenance costs for a Location 2 lock is estimated at $2,600,000 based upon an 7.4 percent discount rate and the annual maintenance cost of $197,000 (Jan 96 price levels).
[4] The present worth of 50 years of routine maintenance costs for the existing 600-foot lock plus a 1,200-foot lock at either Location 1, 3, or 4 is estimated at $5,200,000, based upon an 7.4 percent discount rate and the annual maintenance cost of $394,000 (Jan 96 price levels).

TABLE 14: MAJOR REHABILITATION SUMMARY (1,200-FOOT AND 600-FOOT LOCKS)			
Location and Type	**Major Rehab. Cost**	**Year(s) Incurred**	**Present Worth[1]**
Pile-Founded Locks - 1,200'			
Loc 1, Type C	$3,500,000 [2]	20, 40	$1,000,000
Loc 2, Type B	0	N/A	0
Loc 2, Type C	0	N/A	0
Loc 2, Type R	$25,000,000	25	$4,200,000
Loc 3, Type B	0	N/A	0
Loc 3, Type C	$3,500,000 [2]	20, 40	$1,000,000
Loc 4, Type B	0	N/A	0
Loc 4, Type C	$90,000,000	30	$10,700,000
Rock-Founded Locks - 1,200'			
Loc 2, Type B	0	N/A	0
Loc 2, Type C	$3,500,000	20,40	$1,000,000
Loc 2, Type R	$25,000,000	25	$4,200,000
Loc 3, Type B	0	N/A	0
Loc 3, Type C	$25,000,000	30	3,000,000
Loc 4, Type B	0	N/A	0
Loc 4, Type C	$44,000,000	30	5,200,000
Pile-Founded Locks - 600'			
Loc 1, Type C	$2,300,000 [2]	20, 40	$700,000
Loc 3, Type B	0	N/A	0
Loc 3, Type C	$2,300,000 [2]	20, 40	$700,000
Loc 4, Type B	0	N/A	0
Loc 4, Type C	$60,000,000	30	$7,000,000
Rock-Founded Locks - 600'			
Loc 3, Type B	0	N/A	0
Loc 3, Type C	$17,000,000	30	2,000,000
Loc 4, Type B	0	N/A	0
Loc 4, Type C	$30,000,000	30	3,500,000

Notes:
1. The present-worth values were calculated with a discount rate of 7.4 percent.
2. This would not technically qualify as "Major Rehabilitation" by current guidance; however, the work would need to be done regardless of the funding program.

Operations Costs. Operations costs are for labor, materials, equipment, and supplies for any activity involved with operating the locks. Examples include labor for the lock crew,[5] utility costs (fuel, electricity, phone service, sewer, etc.), office supplies, radios, tools, etc. The same staff and generally the same materials and supplies are needed regardless of the type of lock construction or the size of the chamber (whether 600-foot or 1,200-foot). Therefore, the operations costs are expected to be uniform for each of the lock choices. An exception is for Location 2 locks that result in only a single lock rather than the other options, which result in two operational locks. The annual operations cost for a Location 2 lock is estimated to be $800,000.[6] The annual operations cost for a lock at any other location plus the existing lock is $1,050,000 ($800,000 and $250,000,

[5] The lock crew typically consists of 8 persons per lock site. They rotate in shifts to cover 24 hours per day, 7 days per week of lock operations during the navigation season (typically mid-February to mid-December; however, see next footnote). During the off season, a staff is also maintained at the locks for security, maintenance, and dam operation.

[6] The present worth estimate of 50 years of operations costs for a Location 2 lock is $10,500,000 based upon a 7.4 percent discount rate and the annual operations cost of $800,000 (Jan 96 price levels).

respectively, for the two locks).[7] Since the operations costs do not differ greatly, it is not a significant screening factor, but it needs to be included in the total costs. As noted above under the maintenance and repair costs, the Location 2 lock can be operated at essentially the same costs as the exiting lock at $800,000 annually. This cost is part of the without-project cost, and as a result only the incremental $250,000 annual cost associated with the addition of another facility must be accounted for as a with-project cost.

Economic Impacts to Navigation During Construction.
The economic impact of lock closure caused by construction was investigated. The cost of delaying tows climbs rapidly due to their high operating costs and queuing effect, which is the build-up of numerous tows during lock closure. To quantify the duration of closures due to construction, a construction sequence and schedule for lock measures was developed for those that severely impact navigation. The need for interrupting navigation was addressed for each step in a construction sequence. The delays to navigation would be the highest for construction of a lock at Location 2 and second highest with construction at Location 3. For Locations 1 and 4, the navigation delays are negligible since there is adequate separation from the existing tow maneuvering areas.

The construction schedules place much of the construction work in the winter season. This is one way of reducing delays to navigation during construction since traffic is significantly less in the winter season.[8] This was done to the extent possible, however, some activities cannot be scheduled simultaneously due to sequencing constraints. Drawbacks to winter construction include a decline in construction quality and the additional costs associated with productivity loss from working in colder weather.

Another way of reducing delays to navigation during construction is by constructing large segments (lockwall monoliths, floor slab sections, gate sills, etc.) in an off-site precasting operation, then transporting them to the site for rapid placement by lift-in or float-in methods. These design measures also were incorporated into many of the lock concepts and reflected in the construction cost estimates and construction schedules.

Initial estimates of the costs of delays and closures were made to assess the economic impact to the navigation industry. These initial analyses indicated that these costs are a potentially significant screening factor. Some of the alternatives that are favored due to low first costs may drop in rank compared to other alternatives when the economic impacts to navigation during construction are considered. However, the magnitude of the economic impacts to navigation varies depending upon the economic assumptions and level of congestion at the time construction is undertaken. Assuming that traffic continues to grow with time, then the later the initiation of construction the greater congestion and larger the economic impacts would be to the navigation industry. Construction before traffic levels reach high levels of congestion could reduce the economic impacts.

[7] The present worth estimate of 50 years of operations costs for a lock at any of the other locations plus the existing lock is $13,800,000 based upon a 7.4 percent discount rate and the annual operations cost of $1,050,000 (Jan 96 price levels).

[8] During the winter, navigation usually ceases on the Mississippi River upstream of Quincy, Illinois (L/D 21) and slows at L/Ds 22-25. IWW traffic remains high all year, sometimes even peaking in the winter months. Traffic at Mel Price Lock and Dam and L/D 27 on the Mississippi River also stays high all year, in large part due to the traffic on the IWW, but also due to less severe ice conditions locally.

Summaries of the delays to navigation for each lock concept are shown in Tables 15 and 17 (for pile-founded locks) and Tables 16 and 18 (for rock-founded locks).

TABLE 15: SUMMARY OF DELAYS TO NAVIGATION DURING CONSTRUCTION OF 1,200-FOOT PILE-FOUNDED LOCKS	
Lock Location and Type	**Duration of Delays**
Location 1	
Type C	Negligible
Location 2	
Type B	3 winter continuous closures (90, 90, and 113 days); 550 days of 9-minute delays to upbound exchange tow approaches; and 72 weeks of 8 hr/day x 5 days/week closures
Type C	Same as Location 2, Type B, *plus* the third winter closure would be about 17 days longer
Type R	3 winter continuous closures (97, 96, and 96 days); one 14-day closure during the navigation season; one 8-day closure during the navigation season; and 515 days of 9.4-minute delays to upbound exchange tow approaches
Location 3	
Type B	See note 2.
Type C	4 winter continuous closures (90, 90, 107, and 75 days); 808 days of 9-minute delays to upbound exchange tow approaches; 533 days of 11-min. delays to downbound exchange approaches; one 7-day continuous closure; and 51 weeks of 8-hr./day x 5 days/week closures
Location 4	
Types B and C	Negligible

Notes:
1. The timing of the closures is important in addition to their duration.
2. Not only does the Location 3, Type B lock cost more than the Location 3, Type C lock, but also it has greater impacts to navigation to construct the permanent guidewall. Therefore, the Type B lock was screened out as noted previously.

TABLE 16: SUMMARY OF DELAYS TO NAVIGATION DURING CONSTRUCTION OF 1,200-FOOT ROCK-FOUNDED LOCKS	
Lock Location and Type	**Duration of Delays**
Location 2	
Type B	3 winter continuous closures (90, 90, and 80 days); 550 days of 9-minute delays to upbound exchange tow approaches; and 51 weeks of 8 hr/day x 5 days/week closures
Type C	3 winter continuous closures (90, 129, and 90 days); 504 days of 9-minute delays to upbound exchange tow approaches; one 7-day continuous closure; and 52 weeks of 8 hr/day x 5 days/week closures
Type R	3 winter continuous closures (90, 94, and 96 days); 12 two-day closures during the navigation season and 546 days of 9.4-minute delays to upbound exchange tow approaches
Location 3	
Type B	3 winter continuous closures (90, 90, and 100 days); 610 days of 9-minute delays to upbound exchange tow approaches; 610 days of 11-min. delays to downbound exchange approaches; 21 weeks of 8 hr/day x 5 days/week closures; and 9 weeks of double filling/emptying times
Type C	3 winter continuous closures (90 days each); 640 days of 9-minute delays to upbound exchange tow approaches; 640 days of 11-min. delays to downbound exchange approaches; 28 weeks of 8 hr/day x 5 days/week closures; and 12 weeks of double filling/emptying times
Location 4	
Types B and C	See note 2.

Notes:

1. The timing of the closures is important in addition to their duration.
2. A Location 4 lock could be placed where there are negligible delays to navigation during construction. The Location 4 rock-founded concept lock (developed for Lock and Dam 22 conditions) was placed where it would only remove one dam gate to minimize first costs. With this placement, a Location 4 lock would have impacts similar to those of a Location 3 lock. To avoid these impacts, the lock could be placed farther from the existing lock, thereby displacing an additional tainter gate that would have to be replaced. The additional cost for this is approximately $18 million plus cost adjustments for any quantity increases (that would be determined on a site-specific basis) for constructing in deeper water.

TABLE 17: SUMMARY OF DELAYS TO NAVIGATION DURING CONSTRUCTION OF 600-FOOT PILE-FOUNDED LOCKS

Lock Location and Type	Duration of Delays
Location 1	
Type C	Negligible
Location 3	
Type B	See note 1.
Type C	4 winter continuous closures (90, 90, 107, and 75 days); 550 days of 9-minute delays to upbound exchange tow approaches; 533 days of 11-minute delays to downbound exchange approaches; one 7-day continuous closure; and 18 weeks of 8 hr/day x 5 days/week closures
Location 4	
Type B	Negligible
Type C	Negligible

Notes:
1. Not only does the Location 3, Type B lock cost more than the Location 3, Type C lock, but also it has greater impacts to navigation to construct the permanent guidewall. Therefore, the Type B lock was screened out as noted previously.

TABLE 18: SUMMARY OF DELAYS TO NAVIGATION DURING CONSTRUCTION OF 600-FOOT ROCK-FOUNDED LOCKS

Lock Location and Type	Duration of Delays
Location 3	
Type B	3 winter continuous closures (90, 90, and 100 days); 610 days of 9-minute delays to upbound exchange tow approaches; 610 days of 11-minute delays to downbound exchange approaches; 6 weeks of 8 hr/day x 5 days/week closures; and 9 weeks of double filling/emptying times.
Type C	3 winter continuous closures (90 days each); 606 days of 9-minute delays to upbound exchange tow approaches; 606 days of 11-minute delays to downbound exchange approaches; 13 weeks of 8 hr/day x 5 days/week closures; and 12 weeks of double filling/emptying times.
Location 4	
Type B	See note 2.
Type C	See note 2.

Notes:
1. The timing of the closures is important in addition to their duration.
2. A Location 4 lock could be placed where there are negligible delays to navigation during construction. The Location 4 rock-founded concept lock (developed for Lock and Dam 22 conditions) was placed where it would only remove one dam gate to minimize first costs. With this placement, a Location 4 lock would have impacts similar to those of a Location 3 lock. To avoid these impacts, the lock could be placed farther from the existing lock, thereby displacing an additional tainter gate that would have to be replaced. The additional cost for this is approximately $18 million plus cost adjustments for any quantity increases (that would be determined on a site-specific basis) for constructing in deeper water.

Environmental Resource Impacts. The environmental resource impacts were considered for both site-specific and systemic impacts. The site-specific impacts include the effects of construction such as impacts to particular species, disturbance to or loss of various habitats, increased turbidity, and redirection of normal flow patterns. In general, the locations with the most extreme

environmental impacts were eliminated during the initial screening of lock locations. The remaining locations typically make use of the existing navigation channel or require only slight to moderate channel modifications.

A detailed discussion of the site-specific environmental analysis and development of costs is covered in the *Site Specific Habitat Assessment* report (September 1998). Costs vary rather widely depending on the lock site, location, type, and resources at and around particular locks. For example, at Location 2, average site-specific environmental costs are estimated to range from $530,000 at Lock 24 to $5.3 million at Lock 22 (see Table 19 for information on the other lower lock sites and locations). These costs represent habitat replacement and are only an average of a range identified in the *Site Specific* report. These costs could change during formal mitigation planning. In addition, they do not include costs necessary to address any endangered species or mussel bed impacts. While these cost estimates are specific to the locks and dams shown, they may give a general sense of the site-specific costs at upstream sites (Locks and Dams 11-18).

In contrast with the site-specific costs that are related to the specific location of construction, system environmental costs are instead primarily based on the increase in system traffic and related impacts that any improvements allow over the without-project condition. System environmental impacts are not anticipated to provide deciding information on lock location or types since all of these options provide for similar increases in tow traffic. Both the site-specific and system impacts will be identified in the Environmental Impact Statement (EIS) for the study.

TABLE 19: AVERAGE SITE-SPECIFIC COSTS BY LOCK SITE AND LOCATION

Lock Site	Location 1	Location 2	Location 3	Location 4
Lock 20	-	$1,643,175	$1,106,815	$450,000
Lock 21	-	$4,065,750	$3,995,750	$2,239,500
Lock 22	-	$5,333,344	$5,333,344	$5,453,344
Lock 24	-	$527,940	$597,940	$702,940
Lock 25	$3,123,750	$633,360	$1,058,400	$988,400
La Grange	$5,245,266	$4,834,141	-	-
Peoria	$576,000	$646,000	-	-
Average Costs	$2,981,672	$2,526,244	$2,418,450	$1,966,837

Note: Costs shown above represent average costs based on the ranges identified in the *Site Specific Habitat Assessment* report.

Cultural and Historic Resource Impacts. The Mississippi and Illinois River valleys contain valuable cultural resource sites, primarily early Native American settlements and historic structures of various origins. These cultural resources could potentially be encountered with the construction of new locks. The Historic Properties Work Group, a component of the EIS effort, investigated the cultural and historical resources that could potentially be impacted by lock construction. Efforts conducted as part of the system study indicated that similar impacts are anticipated regardless of the lock location or type implemented. However, because of the large size of the study area, additional investigations would be required during site-specific studies if any lock construction is recommended. Due to uncertainty associated with this type of costs, these impacts were not considered in this screening.

PERFORMANCE BENEFITS

Lockage Time. Where the word "performance" is used, it will generally refer to the lock's capability to perform its basic function of locking boats. Performance levels vary with type and location. For the majority of lockages on the current system, a double lockage is required. The steps of a double lockage were shown in Figure 1.

If lock chambers were as large as the maximum tow size, double lockages would no longer be required. For the Mississippi River and Illinois Waterway, the maximum tow size is also the prevailing tow size, i.e., about 1,200 feet long by 105 feet wide. Constructing a 1,200-foot-long lock at a site would eliminate a number of the lockage steps currently required, resulting in a very large reduction in lock transit time. The resulting lockage would then be a single lockage, having the steps indicated in Figure 2. In addition, constructing a new lock at Locations 1, 3, or 4 would result in the site having two locks available. This would not only allow for improvement in the individual lockage times, but also would provide some potential for multiple lockages to occur simultaneously. An additional benefit of multiple chambers is the ability to still have one working chamber if accidents occur or repairs are needed. However, considerable limitations are anticipated in operating two chambers simultaneously. This would likely require actions such as traffic staggering and size limitations.

Table 20 gives an overview of the existing lockage times and estimated new lockage times for 1,200-foot tows at new or extended locks. In general, eliminating the double lockage steps saves a tow approximately 60 minutes (eliminates the need to extract first cut, turn back chamber, enter and lock a second cut, and remake; see time differences in Figures 1 and 2). However, due to the extra time needed to enter and exit a longer chamber and the longer filling and emptying times associated with the greater water volumes, somewhat smaller savings are anticipated as shown below. In contrast with existing double lockages, existing single lockages (tows less than 600 feet) would actually experience a minor increase in lockage time because they would be transiting an additional 600 feet of chamber and because the filling and emptying of the large volume of water requires some additional time. This increased time is likely to be offset by the greater ability to lock multiple small tows, recreational craft, and combinations of vessels together. In contrast, a new 600-foot chamber is not expected to provide much faster lockages per tow. The principal improvement is simply having an additional chamber available. The small time differences shown in Table 20 are generally related to filling and emptying times and approach and exit conditions. Filling and emptying times and approaches and exits are anticipated to be somewhat worse at Location 1 and 2 locks.

The Type 2R lock was included to provide a more complete set of alternatives. For screening purposes, the Type 2R lock time savings assumes some channel improvement work will be done above the lock but that the upstream guidewall will not be extended. Channel improvements, if not included with the Type 2R designs, could be constructed under existing authority (see the Engineering Appendix and Addendums for cost and time savings estimates without channel improvements). However, due to slower filling and emptying associated with not extending the culverts, the 2R lock option is 2 to 4 minutes slower than other Location 2 lock types.

TABLE 20: AVERAGE LOCKAGE TIME AND TIME SAVINGS OF 1,200-FOOT AND 600-FOOT LOCKS			
Location/Type	**Avg Lockage Time[1,3] (min)**	**Avg Time Savings[1,3] (min)**	**Number of Sites[2]**
Existing 600'	103	-	15
1,200 ft Locks			
1C	59	43	4
2B	54	49	15
2C	57	46	5
2R[4]	57	46	15
3B	59	44	4
3C	53	49	12
4B	52	50	11
4C	52	50	11
600 ft Locks			
1C	107	-4	4
Loc 2	N/A	N/A	N/A
3B	106	-3	4
3C	100	3	12
4B	99	4	11
4C	99	4	11

Notes:
[1] The average values assume equal numbers of fly, exchange, and turnback lockages. However, in actuality these are not equal. The time savings shown are from tows currently needing double lockages (e.g., greater than 600 feet total length). Smaller tows can lock in significantly less time. For numbers shown, turnback approaches are assumed to take place during lockage of the previous tow.
[2] Number of sites evaluated is shown out of a possible 15. While Lock 19 was also evaluated, it already has a 1,200-foot chamber and as a result would skew the results.
[3] There is no difference in the performance between Type B and C at any location; differences shown only relate to the fact that averages are for different numbers of sites.
[4] Taking into consideration only the same locations, Type 2R locks are anticipated to be 2 minutes slower for fly and exchange lockages, and 4 minutes slower for turnback lockages than other Type 2 locks.

In comparing lock measures of the same lock length at the same site, most of the lockage steps are of equal duration across measures. The times for entering the chamber, gate operation, and exit are essentially constant. There could be differences in the durations of two of the remaining lockage steps: approach times and filling and emptying times. There are different influences on these two lockage components. The approach time varies from one site to another and could be different at different lock locations at a given site. In addition, the cost of the required channel work will vary by lock location, and this is accounted for in the lock cost estimates. The filling and emptying time depends on lock size and lock type (due to the design of the filling and emptying systems). The filling and emptying times also vary by the lock lift (or head differential); however, this is equal among the lock alternatives at a given site. The quantification task for evaluating lockage time distinctions between large-scale alternatives was therefore focused on the approach times and filling and emptying times for the remaining lock measures. These estimates follow.

Approach Time Estimation. The approach times were developed based upon the navigation modeling results for Lock and Dam 22 and the approach time improvements estimated for analyzing small-scale measures. The focus of approach analysis efforts was on downbound approaches, since upbound approaches tend to be less variable and less troublesome. It was determined that the maximum approach time improvement that could be attained at the existing Lock 22 is about a 35 percent timesaving. Since Lock and Dam 22 has the worst existing downbound approach times of the locks in the study, the 35 percent improvement experienced in the model study estimated to be the maximum improvement achievable at other lock sites. At the opposite extreme from Lock and Dam 22, it is unlikely that any improvement could be made to the

approach at Lock and Dam 13. The magnitude of the approach time improvements applied at each site, as part of these large-scale improvements, is very similar to the channel improvements used in the small-scale analysis that was used as a reference.

Table 21 summarizes the approach savings by lock location. Savings are the same for all lock types at each location. The one exception is Location 2R; this type has less savings than 2B and 2C, as shown in the following tables. This difference is due to the fact that the 2R lock does not include approach improvements unless required. However, there are differences based on the approach type (fly or exchanges). Turnback approaches are not shown since it is assumed that they can make their approach during the locking of the previous tow. While some upbound time savings were identified for the various locations, they averaged roughly 1 minute, with most locations having essentially the same benefits.

TABLE 21: DOWNBOUND APPROACH TIMES AND TIME SAVINGS BY LOCK LOCATION (MINUTES)

Lock Location	Fly		Exchange	
	Time Estimate[2]	Time Savings[2]	Time Estimate[2]	Time Savings[2]
Existing[1]	27.9	-	21.8	-
Loc 1	18.3	9.6	16.8	5.0
Loc 2[3] (w/approach)	25.1	2.8	19.7	2.1
Loc 2[4] (wo/approach)	27.2	0.7	21.1	0.7
Loc 3	20.8	7.0	15.8	6.0
Loc 4	20.7	7.1	15.2	6.6

[1] Existing times represent averages for 1990 at UMR Locks 11-25 and IWW Locks Peoria and La Grange.
[2] New Location 1-4 estimated times and savings based on engineering study efforts. Approach savings are the same for each location regardless of chamber size (600 or 1,200 feet). No differences in turnback approach times are anticipated.
[3] Location 2 savings are associated with the 2B and 2C alternative, including minor channel improvements at most sites.
[4] Location 2R savings assuming only minimal/required channel improvements are made. The limited savings shown are related to improvements at Locks 14, 15, and 25, which are anticipated to be required.

Filling and Emptying Time Estimation. Filling and emptying times were computed for each location and type using the Tennessee Valley Authority (TVA) program TFSIM. Existing filling and emptying times are approximately 4 to 5 minutes each. These times are anticipated to increase by 3 to 7 minutes due to the greater volume of water. The slowest times are associated with Location 1 that uses a single filling culvert and that must empty into the lower approach due to the lock location and thereby reduce performance. Location 2 times are also somewhat slower because a Type 2R lock would not include an extension of the filling and emptying system. (See the Engineering Appendix, Addendum G for final site-specific filling and emptying time estimates.)

TABLE 22: FILLING AND EMPTYING TIMES AND DIFFERENCES BY LOCK LOCATION (MINUTES)

Lock Location	Filling		Emptying	
	Time Estimate[2]	Additional Time	Time Estimate[2]	Additional Time
Existing[1]	4.1	-	4.5	-
Loc 1[3]	10.6	6.5	11.5	7.0
Loc 2[4]	9.0	4.9	11.1	6.6
Loc 3	7.4	3.3	10.0	5.5
Loc 4	7.1	3.0	9.1	4.6

[1] Times represent 1990 averages for UMR Locks 11-25 and IWW Locks Peoria and La Grange at 600-foot chambers.
[2] New Location 1-4 estimated times and savings based on engineering study efforts and 1,200-foot chambers.
[3] Location 1 has slower times due to the design and use of a center fill system that empties into the lower approach.
[4] Location 2 refers to 2 R and slower times associated with not extending the filling and emptying system.

Disposition of the Existing Lock. Another aspect of with-project performance is the disposition of the existing lock after a new lock is constructed at another location at the same lock and dam site. For a lock extension, the existing lock is incorporated into the new lock and there is no second lock; however, for construction at each of the other lock locations, several outcomes are possible concerning the existing lock:

1. Unobstructed approaches - lock remains functional for both recreational and commercial traffic with tows up to 1,200 feet long.
2. Partially obstructed approach - lock available only to recreational traffic (and possibly tows with size restrictions).
3. Hazardous approach conditions - lock unusable for either commercial or recreational craft.

The above outcomes are over simplified, but they present a few of the possibilities. The necessary provisions to ensure navigation safety must be determined on a site-specific basis and would probably require navigation modeling. Subjective assessments were made of the with-project functioning of the existing lock after a second lock is constructed, see Table 23.

TABLE 23: USE OF EXISTING LOCK AFTER NEW LOCK CONSTRUCTION

Lock Site	With Location 1 Lock		With Location 3 Lock		With Location 4 Lock	
	Upstream	Downstream	Upstream	Downstream	Upstream	Downstream
L/D 11	n/a	N/A	Full size tows	Full size tows	N/A	N/A
L/D 12	N/A	N/A	(No restrict.)	Recr./Single	(No restrict.)	Recr./Single
L/D 13	N/A	N/A	Full size tows	Full size tows	Full size tows	Full size tows
L/D 14	N/A	N/A	N/A	N/A	Recr./Single	Recr./Single
L/D 15	N/A	N/A	Recr./Single	Recr./Single	N/A	N/A
L/D 16	N/A	N/A	(No restrict.)	Recr./Single	(No restrict.)	Recr./Single
L/D 17	Full size tows	Full size tows	(No restrict.)	Recr./Single	(No restrict.)	Recr./Single
L/D 18	N/A	N/A	(No restrict.)	Recr./Single	Full size tows	Full size tows
L/D 19	N/A	N/A	Full size tows	Full size tows	N/A	N/A
L/D 20	N/A	N/A	(No restrict.)	Recr./Single	(No restrict.)	Recr./Single
L/D 21	N/A	N/A	Full size tows	Full size tows	Full size tows	Full size tows
L/D 22	N/A	N/A	Full size tows	Full size tows	Full size tows	Full size tows
L/D 24	N/A	N/A	Full size tows	Full size tows	Full size tows	Full size tows
L/D 25	Recr./Single	(No restrict.)	Recr./Single	Recr./Single	Recr./Single	Recr./Single
Peoria	Full size tows	Full size tows	N/A	N/A	N/A	N/A
La Grange	Full size tows	Full size tows	N/A	N/A	N/A	N/A

Notes:
1. "N/A" is used where the new lock location has been screened out and is not applicable.
2. "Full size tows" means that 1,200-foot-long tows could safely make an approach to the existing lock. However, traffic would have to be staggered between the two locks to avoid interference in the approaches and exits.
3. "Recr./Single" means that, after construction of a new lock, the existing lock could only be used by recreational craft and tows smaller than 600 feet long (i.e., tow size that pass in a single lockage).
4. "(No restrict.) is used to indicate that there are no restrictions on tow size on either the upstream or downstream side (as indicated); however, the approach on the other side of the lock limits the maximum tow size that will be able to use the existing lock (for transit in both directions).

An approach could be “obstructed” by the placement of the upstream or downstream guidewalls (or guardwalls) of the new lock, or even by a lockwall of the new lock. In some cases, this can be overcome by opening up the approach by cutting back the riverbank. Another way in which the approach could be obstructed is by having conflicting approaches for the existing and new locks. In this condition, tows could only approach one lock at a time from the same direction. In fact, this would be the case for each of the new locks due to their close proximity to the existing locks. Delays to either lock could be minimized by sequencing measures that would be more flexible with two locks than with one. Causes of “hazardous” approach conditions include severe obstruction, adverse currents, or inadequate water depths. Some of the approaches were severely obstructed, but channel improvement measures were included in the cost estimates to allow access for tows up to 600 feet long to use each of the existing locks.

For each of the lock concepts at Location 3 (except at Lock 19), the existing intermediate lockwall filling and emptying culvert would be dedicated to the new lock. This would mean that the existing lock would be filled and emptied only from the landside, with resultant slower filling and emptying times for the existing lock. These slower times would occur because the flow area is cut in half and because filling from one side of a lock tends to shove tows over towards one wall. Valve opening and closing times would have to be lengthened to keep hawser forces at an acceptable level and to minimize turbulence. Nevertheless, this is a feasible method to fill a lock because it has been used during instances of culvert valve malfunction.

SECTION 4 - SECONDARY SCREENING

This section takes the quantitative numbers developed and discussed in the previous section and combines them with additional information learned from preliminary system model runs to conduct a secondary screening. This screening was based primarily on quantitative factors and cost effectiveness comparisons. However, the expertises of the various technical disciplines were used as well.

APPLICATION OF COST AND PERFORMANCE INFORMATION

This section builds on the new lockage times and costs of the various remaining 1,200-foot and 600-foot lock options, as described in the previous section. Additional efforts were taken to include estimates of impacts to navigation and site-specific environmental costs for inclusion prior to a secondary screening. While system environmental and cultural impact costs were not included at this point, the magnitude of these costs was not anticipated to provide deciding information as to the selection of the most economically efficient location or type. These impacts will be considered as part of the alternative plan analysis before a study recommendation is made.

The analysis then extended the new lockage time of the new or extended lock plus the value of the existing lock, if available, to obtain a total potential ability to lock boats. This number can then be compared with the existing locks' ability to lock boats and an incremental benefit identified. The following paragraphs discuss how these numbers were developed and their limitations.

Impacts to Navigation Cost Estimates

Estimates of the impacts to navigation were included, in addition to the first costs of construction and the present worth of increased operations and maintenance and rehabilitation needs discussed in the previous section. To determine the impacts to navigation cost of implementing various options, preliminary system economic model runs were needed.

The preliminary runs, conducted in November 1998, showed that impacts to navigation for Type 2R locks averaged roughly $12 million per lock site for the five lower locks on the UMR. From this value, an estimate of the impacts to navigation for the other lock options was calculated based on relative hours of delay caused by construction of the other locations and types. Total delay hours were calculated using the information from Tables 15 through 18 in the previous section. For example, Location 2B has roughly twice the delay hours as a Location 2R at rock-founded sites and as a result could be estimated to cause approximately twice the delay costs.

TABLE 24: ESTIMATED IMPACT TO NAVIGATION HOURS, PERCENT AVAILABLE AND COST					
Lock Location and Type[1]	**Total Hours of Availability Lost [2,3]**	**Percent Reduction over 3-Year Const.[4]**	**Percent Lock Available[4]**	**Ratio of Delays to 2R Const.[6]**	**Estimated Cost Based on 2R Lock ($Mil)**
Rock-founded 1,200 ft locks					
2B	2205	11%	89%	2.10	$25
2C	3335	17%	83%	3.18	$38
2R	987	5%	95%	0.94	$11
3B	1571	8%	92%	1.50	$18
3C	1715	9%	91%	1.63	$20
4B[5]	1571	8%	92%	1.50	$18
4C[5]	1715	9%	91%	1.63	$20
Pile-founded 1,200 ft locks					
2B	3597	18%	82%	3.42	$41
2C	4005	20%	80%	3.81	$46
2R	1145	6%	94%	1.09	$13
3C	3054	15%	85%	2.91	$35
Rock-founded 600 ft locks					
3B	971	5%	95%	0.92	$11
3C	924	5%	95%	0.88	$11
4B[5]	971	5%	95%	0.92	$11
4C[5]	924	5%	95%	0.88	$11
Pile-founded 600 ft locks					
3C	1656	8%	92%	1.58	$19

[1] Locations and types not shown have essentially 100 percent availability of the existing lock during construction (Location 1 and Location 4 at pile-founded sites). Construction occurring during the 90-day winter closure or away from the lock approaches was not assumed to impact navigation (except at Peoria and La Grange on the IWW, which do not have a winter closure period).
[2] For calculating delays in approaches, assumed 5 tows traveling each way per day (10 tows per day), 2 of which are upbound exchanges and 2 downbound exchanges. The other 6 lockages are anticipated to be turnback lockages. Even with greater numbers of tows, it is unlikely that the number of exchanges would increase above 4 total per day.
[3] Impacts to filling and emptying assumes 4 minutes per cycle (doubling would add 4 additional minutes) times the average of 2 cycles per lockage, times 10 lockages per day.
[4] The percentage reductions and available are based on delays occurring during a 275-day/year navigation season and 3-year construction schedule. This assumes no impacts associated with lock closures during the 90-day winter. This understates costs at some of the lower UMR lock sites where limited navigation occurs year round. Peoria and La Grange costs were estimated separately based on 365 days and inability to avoid impacts to navigation during the winter. This results in an increase of impacts to navigation of 6,480 hours (90 days x 24 hours x 3 years). Increasing costs by $74 million over UMR pile-founded 2B and 2R locks.
[5] At rock-founded Location 4 locks, the impacts depend on the placement of the lock in the gated section of the dam. These impacts could be eliminated by locating the lock farther out in the gated section of the dam, at an additional first cost of $18 million. In instances where the impacts to navigation are greater than $18 million, the location would be moved to eliminate the impacts and the additional first cost incurred.
[6] The ratios are compared to a weighted average of 3 rock-founded and 2 pile-founded 2R locks, which is equivalent to roughly 1,050 hours of lost lock time and $12 million in impact costs per lock based on initial system economic model runs.

The delays associated with having just 600-foot chambers versus a 1,200-foot chamber also impact navigation cost. Even without a queue, the process of using the 600-foot chamber takes roughly 45 to 50 minutes longer, which is in essence an impact to navigation. As a result, there is an added cost of using 600-foot chambers. However, rather than a system delay associated with a total lock shut down, the delay cost averages roughly $300 per hour per tow. This still results in a significant annual cost. For example, assuming 2,500 double lockage tows per year and an additional 45 minutes of time required to lock each double lockage tow, shows an increase of 1,875 hours of

delay time annually. This equals roughly $560,000 in delays to navigation, even without considering the cumulative delays that occur if other tows are waiting in queue during these slower lockages. This cost is an annual cost that will be incurred each year and will increase as the number of boats increases. In contrast, the costs shown above are one-time delay costs associated with the 3-year construction period. However, one benefit of having two locks is that impacts to navigation could be reduced during a closure of one lock since boats could still utilize the other lock.

Site-Specific Environmental Cost Estimates

The section on the quantification of costs summarized the environmental costs developed in the *Site-Specific Habitat Assessment* for the lower seven lock sites. A brief evaluation of these costs, compared with the other lock costs, reveals that from a strictly monetary standpoint these environmental costs are not of sufficient magnitude to represent a significant screening factor between the various locations and types. However, these costs were included in order to understand better the approximate total costs of the various large-scale options. To provide costs for all 16 sites, rough estimates were made of the costs at Locks 11-19, the sites not monetized in the *Site-Specific Habitat Assessment* report. These costs were developed using the qualitative analysis of these sites completed as part of the site-specific effort and then estimating habitat replacement costs relative to the values developed for the lower seven sites. In addition, the site-specific environmental costs of 600-foot chambers were estimated at 75 percent of the 1,200-foot locks for the same location at the same lock site. While further site-specific cost refinements would be necessary as part of the site-specific design process if improvements are recommended, the method used is considered to be a reasonably accurate estimate of cost.

The costs shown in the following tables indicate the habitat replacement costs, but they do not fully include other factors such as endangered species, mussel beds, or socio-economic impacts that could potentially impact environmental costs or lock placement decisions. These factors were considered qualitatively and were considered in the screening. At these additional sites, some of the more significant issues are listed below. These factors were taken into account to the extent possible in developing the site-specific costs:

Lock 24: Socio-economic cost not considered but upstream lock construction will be considered in the site-specific feasibility study.

Lock 20: Socio-economic impacts related to development along the Canton, Missouri, riverfront.

Lock 17: A new lock at Location 1 would result in extensive loss of bottomland hardwood forest habitat that should be avoided or minimized to the extent possible.

Lock 16: Proposed realignment of the upstream approach channel with 2B and Locations 3 and 4 locks would result in considerable impact to the side channel and diverse backwater aquatic habitats.

Lock 14: New lock construction presents considerable concern over the potential to degrade channel border and wetland habitat and damage the historic side channel area through lock construction.

Lock 12: Any new or extended lock construction presents significant potential for socio-economic impacts by impairing the view of the river from historic downtown Bellevue, Iowa, and eliminating access to a public boat ramp, as well as potentially impacting a locally important sport fishing area.

Performance of New Facility

In order to compare the various options, the performance of the locks was stated in comparable terms by starting with the average lockage time at each site. The average lockage times were based on equal portions of fly, turnback, and exchange lockages. While this is not likely to occur, a sensitivity analysis that examined higher percentages of turnback and exchanges did not reveal significant differences in average lockage times or differences in the relative merits of the various lock options. As a result, the simplified assumption of equal distribution of the various lockage types was used for this analysis.

Using the new lockage times and resulting presence of either one or two chambers, an estimate was made of the approximate ability to process double lockage tows (e.g., tows longer than 600 feet; these will become singles with 1,200-foot locks). This number is only hypothetical since it does not look at various tow sizes or take into account the numerous limitations in locking efficiency due to flow conditions, adverse weather conditions, equipment failure, etc. It does not take into account the limitations of jointly using two chambers (e.g., the existing lock in combination with Locations 1, 3, and 4 locks were not shown as limited in their ability to lock boats due to outdraft and conflicts in the approach and exits, which are likely to occur). The analysis also did not include the value of having two chambers operating during the limited periods of periodic closure. The focus of this evaluation was to summarize the performance benefits in comparable terms.

For example, to compare the relative efficiency of having one 1,200-foot chamber versus two chambers, an estimation of the ability to lock boats per day was calculated. For example, if a new 1,200-foot lock extension has an average lockage time of 57 minutes, then roughly 25 boats could be locked per day (1,440 minutes per day/57 minutes per lockage = 25.3 lockages per day). Similar calculations were made for locations that would result in two chambers at a lock site. However, when two locks would be available, the total number of tows per day would be the total of the two individual locks. For example, if the same 1,200-foot chamber were available in combination with an existing lock, which could lock 14 boats, then together they could lock roughly 40 boats per day (assuming no conflicts, which are likely to occur).

From this information, a percentage increase in capacity could be approximated by dividing the new boats per day by the former ability to lock boats. For example, if the new facility has one extended 1,200-foot lock allowing approximately 25 boats to lock per day while the former facility could only lock 14, then the new facility would have approximately 181 percent of the former lock's ability to process tows, representing an incremental increase of 81 percent. Again, as stated above, this method likely overstates the actual lock capacity, but it does provide a uniform method to compare the various options.

Cost Effectiveness Category

One additional category was developed using the cost and performance information discussed above. The final column in Table 25 shows the percentage increase in efficiency per million dollars of improvement cost. This column in essence allows an incremental cost comparison of the various measures. Continuing with the example above, the 181 percent increase in capacity was divided by the cost of the improvement in millions, $134. This creates a cost effectiveness

category, percent increase in the ability to lock tows per million dollars of expenditure, which allows a comparison between the relative effectiveness of the various measures.

While the summary table provides a good indication of the relative cost, performance, and cost effectiveness, the screening was conducted looking at each lock site individually due to site-specific differences and the variation in number of sites included in the averages.

TABLE 25: SUMMARY OF NEW LOCK COSTS, PERFORMANCE, AND COST EFFECTIVENESS

Lock Type	New Lockage Time[1]	Costs w/o Impacts to Nav or Env ($1,000)	Estimated Impacts to Nav ($1,000) [2]	Site-Specific Env. Cost ($1,000) [3]	Total Cost ($1000)	No. of Sites	Tow/Day New Lock[4]	Tow/Day Existing Lock[4]	Total Tow/Day Facility[4]	Percent Increase Tow/Day	Percent Increase/ $1 mil
1,200-Foot Locks											
1C	59	$176,150	$0	$4,111	$180,261	4	24	14	38	275	1.53
2B	54	$138,309	$45,533	$2,679	$186,521	15	27	0	27	192	1.08
2C	57	$114,664	$38,000	$2,621	$155,285	5	25	0	25	189	1.22
2R	57	$110,105	$22,200	$1,819	$134,124	15	25	0	25	181	1.41
3B	59	$157,798	$18,000	$1,910	$177,707	4	25	13	38	286	1.63
3C	53	$188,869	$30,000	$2,466	$221,335	12	27	14	41	293	1.34
4B	52	$261,273	$6,545	$2,830	$270,649	11	28	14	42	294	1.10
4C	52	$257,791	$6,545	$2,830	$267,167	11	28	14	42	294	1.12
600-Foot Locks											
1C	107	$164,425	$0	$3,083	$167,508	4	13	14	27	197	1.18
3B	106	$132,798	$11,000	$1,432	$145,230	4	14	13	27	203	1.42
3C	100	$155,111	$16,333	$1,985	$173,429	12	14	14	29	202	1.19
4B	99	$221,727	$4,000	$2,270	$227,997	11	15	14	29	202	0.89
4C	99	$229,182	$4,000	$2,270	$235,452	11	15	14	29	202	0.87

Notes:
[1] Average lockage time (minutes) is based on equal percentages of fly, exchange, and turnback lockages. The numbers shown assume turnback approaches occur during the lockage of the previous tow, this may understate overall lockage time slightly.
[2] Costs for impacts to navigation are estimated based on initial model runs indicating costs of $12 million per lock associated with construction of 2R locks. Costs for other sites were estimated based on relative hours of unavailability compared to the 2R locks. The high average value shown above is skewed by the much higher costs of Location 2R locks at Peoria and La Grange Locks on the IWW.
[3] Environmental costs for Locks 20-25, Peoria, and La Grange are the average of the ranges provided in the *Site Specific Habitat Assessment* report. The values for the other sites were estimated based on qualitative analysis of the sites and relative impacts compared to the seven sites which had specific costs identified. The environmental costs associated with the 600-foot locks were estimated at 3/4 the cost of the 1,200-foot lock options, but were not looked at in detail.
[4] The tows per day are based on the average time to lock double lockage tows. The percentage increase in capacity is likely overstated for locations with 2 locks, since no reduction in efficiency of the existing lock was incorporated in the evaluation.

SECONDARY SCREENING PROCESS

Using the information summarized in the table above and presented for each lock site in Table 26, a secondary screening was conducted on a site-by-site basis. Due to the differences in cost effectiveness of the various measures, it is relatively easy to screen a number of the lock types, lengths, and locations. In addition to the cost effectiveness numbers, the study team utilized all other available engineering, economic, and environmental information and professional judgment in conducting the screening.

Cost effectiveness provides a valid technique to compare the relative benefits of various measures. However, it is best used to screen between measures that have either similar benefits or costs. For example, it is best used when measures provide similar increases in efficiency at differing costs. It

is not as effective when the measures have greatly different costs and performance. In cases where a measure provides significantly greater benefits (ability to lock boats) at a greater cost, even if the efficiency is somewhat lower, the system economic model is needed to determine if the extra benefits are worth the cost.

As the above table shows, virtually all the large-scale improvement options would nearly double the ability to lock large tows (roughly a 200 percent increase). In the case of a new 1,200-foot lock that still allows the existing 600-foot lock to be used, the increase would be closer to a tripling (300 percent increase). As a result, two separate cost-effectiveness comparisons were made. First, measures providing approximately a doubling in total ability to lock tows per day were evaluated; then a separate comparison was made between the options that would more nearly triple the ability to lock tows per day.

Starting with the lowest cost option, a comparison shows that at Lock 24 a 2R lock would increase efficiency by roughly 189 percent over the existing lock alone (an incremental increase of 89 percent). This results in about a 1.44 percent increase in efficiency per million dollars of cost. Comparing this efficiency with the efficiency of other Location 2 locks (202 percent increase for both 2B and 2C at an increased efficiency of 1.27 and 1.18 per million dollars, respectively), it is possible to eliminate the Locations 2B and 2C lock options based on cost efficiency. These lock options provide very similar benefits to the 2R lock but do so at a higher cost. This same process was followed at each lock site with the same result.

Again using the 2R locks as a comparison, the analysis indicates that the addition of another 600-foot lock provides relatively comparable lockage ability (roughly double lockages per day). However, the overall ability to lock with two 600-foot chambers is potentially greatly overstated because there will be inefficiencies due to conflicts in the approach path. Despite this potential overstatement, it is clear that at virtually all sites a 2R lock would provide similar benefits more efficiently than any of the 600-foot lock options. In addition, it is likely that the most efficient of the 600-foot lock options, Location 3, would have the most significant reductions in efficiency because both chambers could not be used to full effectiveness at the same time. Implementing 600-foot locks also would result in the continuation of time-consuming and highly variable double lockages with associated greater operations, risks, and inefficiencies. A rough estimate of the impacts to navigation associated with these slower lockage times of $560,000 per year was discussed above but was not included in the cost-effectiveness calculations. The only offsetting benefit would be the redundancy in having two chambers to offset losses during closures of one of the chambers. However, unplanned closures are rare and typically do not result in lock closures of significant magnitude. Based on this information, the 600-foot lock options were eliminated from consideration. The only UMR site where the cost efficiency appears higher for the 600-foot locks over the 2R lock extension is at Lock 22. However, based on the additional qualitative factors discussed above, the 600-foot lock options were eliminated here as well.

The IWW sites are an exception to the above discussion due to the much higher impacts to navigation costs associated with extending the existing lock, Location 2. Year-round navigation on the IWW allows no regular winter closure period for construction with minimal impacts to navigation. One possible way to avoid these high impacts would be to construct all or part of the extension during periods of open pass (which occur approximately 35 and 45 percent of the time, respectively, at Peoria and La Grange). During these periods, delays to tows potentially could be minimized or eliminated. If these high impacts to navigation are included (as shown in tables) for the 2R option, 600-foot or 1,200-foot Location 1C locks would dominate the 2R lock extension option. However, due to uncertainties in how to address impacts to navigation, both locations (1C

and 2R 1,200-foot locks) will be carried forward through secondary screening. In addition, due to the considerable limitation in operating two 600-foot locks and similarities in cost to the 1,200-foot 1C lock, the option of constructing a new 600-foot lock at Location 1C was eliminated.

Next, the various options that would yield a new 1,200-foot option while leaving the existing lock were compared. These options included the 1C, 3B, 3C, 4B, and 4C lock options. This evaluation revealed that where they are available (only four sites) Location 1C locks appear to be the most cost effective. Of the options available at virtually all sites, Location 3 locks dominate Location 4 locks. The Location 3 locks provide essentially the same benefits as Location 4 locks at an average cost of approximately $50 million lower, resulting in a higher cost effectiveness. This allowed Location 4 locks to be screened out in favor of Location 3. An exception is Lock 14, which does not have a Location 1 or 3 improvement option.

In most cases, the cost differences between the Type B and Type C lock options at specific lock sites is not significant. The differences of a few million are on costs of well over $100 million. However, to reduce the number of measures, only the lower cost type was retained.

In cases where traffic growth is assumed to be relatively modest, a lower cost 2R lock extension may be preferable to other locations that would provide two locks. In contrast, if traffic and delays are considered to grow more significantly into the future, it may be desirable to implement locks at Locations 1, 3, or 4. Many of these items are best handled in the context of the system model and cannot be adequately screened on a simple cost-effectiveness basis. The only exceptions are the IWW sites since at these locations the Type 1C lock presents lower total costs and as a result could be used to eliminate the Type 2R lock. However, as discussed previously, due to the potential associated with 2R locks to reduce impacts to navigation, both 1C and 2R 1,200-foot locks were carried forward through the secondary screening.

TABLE 26: SITE-SPECIFIC SUMMARY OF NEW LOCK COSTS, PERFORMANCE, AND COST EFFECTIVENESS

Lock Site	Lock Location Type		Lock-age Time[1]	Costs w/o Impacts to Nav or Env ($1000)	Estimated Impacts to Nav ($1000) [2]	Site-Specific Env. Cost ($1000) [3]	Total Cost ($1000)	Tow/Day New Lock	Tow/Day Existing Lock	Total Tow/Day Facility	Percent Increase Tow/Day	Percent Increase/ $1 mil
LaGrg.	Ex. Lock		103									
1,200ft	Loc 1	C	60	$169,400	$0	$5,245	$174,645	24	14	38	271	1.55
	Loc 2	B	57	$152,050	$115,000	$4,834	$271,884	25	n/a	25	180	0.66
		R	60	$114,250	$87,000	$4,834	$206,084	24	n/a	24	172	0.84
600 ft	Loc 1	C	105	$155,100	$0	$3,934	$159,034	14	14	28	199	1.25
Peoria	Ex. Lock		107									
1,200ft	Loc 1	C	64	$173,500	$0	$576	$174,076	23	14	36	267	1.53
	Loc 2	B	59	$150,000	$115,000	$646	$265,646	24	n/a	24	181	0.68
		R	62	$121,250	$87,000	$646	$208,896	23	n/a	23	173	0.83
600 ft	Loc 1	C	112	$160,200	$0	$432	$160,632	13	14	26	195	1.22
25	Ex. Lock		101									
1,200ft	Loc 1	C	56	$184,000	$0	$3,124	$187,124	26	14	40	279	1.49
	Loc 2	B	51	$161,200	$41,000	$633	$202,833	28	n/a	28	198	0.97
		R	54	$124,400	$13,000	$633	$138,033	27	n/a	27	188	1.36
	Loc 3	C	50	$202,930	$35,000	$1,058	$238,988	29	14	43	301	1.26
	Loc 4	B	50	$287,930	$0	$988	$288,918	29	14	43	302	1.04
		C	50	$264,630	$0	$988	$265,618	29	14	43	302	1.14
600ft	Loc 1	C	108	$173,400	$0	$2,343	$175,743	13	14	28	194	1.10
	Loc 3	C	99	$167,630	$19,000	$794	$187,424	15	14	29	202	1.08
	Loc 4	B	99	$236,930	$0	$741	$237,671	15	14	29	202	0.85
		C	99	$229,930	$0	$741	$230,671	15	14	29	202	0.88
24	Ex. Lock		107									
1,200ft	Loc 2	B	53	$133,830	$25,000	$528	$159,358	27	n/a	27	202	1.27
		C	53	$132,830	$38,000	$528	$171,358	27	n/a	27	202	1.18
		R	56	$120,230	$11,000	$528	$131,758	26	n/a	26	189	1.44

TABLE 26: SITE-SPECIFIC SUMMARY OF NEW LOCK COSTS, PERFORMANCE, AND COST EFFECTIVENESS												
Lock Site	Lock Location Type		Lock-age Time[1]	Costs w/o Impacts to Nav or Env ($1000)	Estimated Impacts to Nav ($1000)[2]	Site-Specific Env. Cost ($1000)[3]	Total Cost ($1000)	Tow/Day New Lock	Tow/Day Existing Lock	Total Tow/Day Facility	Percent Increase Tow/Day	Percent Increase/ $1 mil
	Loc 3	B	54	$201,730	$18,000	$598	$220,328	27	13	40	298	1.35
		C	54	$198,730	$20,000	$598	$219,328	27	13	40	298	1.36
	Loc 4	B	51	$259,730	$18,000	$703	$278,433	28	13	41	307	1.10
		C	51	$268,430	$18,000	$703	$287,133	28	13	41	307	1.07
600ft	Loc 3	B	105	$168,730	$11,000	$448	$180,178	14	13	27	201	1.12
		C	105	$158,730	$11,000	$448	$170,178	14	13	27	201	1.18
	Loc 4	B	105	$231,730	$11,000	$527	$243,257	14	13	27	201	0.83
		C	105	$246,730	$11,000	$527	$258,257	14	13	27	201	0.78
22	Ex. Lock		112									
1,200ft	Loc 2	B	63	$121,830	$25,000	$5,333	$152,163	23	n/a	23	177	1.17
		C	63	$115,830	$38,000	$5,333	$159,163	23	n/a	23	177	1.11
		R	68	$101,230	$11,000	$3,083	$115,313	21	n/a	21	165	1.43
	Loc 3	B	64	$142,030	$18,000	$5,333	$165,363	22	13	35	274	1.66
		C	64	$138,030	$20,000	$5,333	$163,363	22	13	35	274	1.68
	Loc 4	B	63	$206,830	$18,000	$5,453	$230,283	23	13	36	278	1.21
		C	63	$200,030	$18,000	$5,453	$223,483	23	13	36	278	1.24
600ft	Loc 3	B	107	$120,030	$11,000	$4,000	$135,030	13	13	26	204	1.51
		C	107	$109,030	$11,000	$4,000	$124,030	13	13	26	204	1.65
	Loc 4	B	107	$194,830	$11,000	$4,090	$209,920	13	13	26	204	0.97
		C	107	$189,330	$11,000	$4,090	$204,420	13	13	26	204	1.00
21	Ex. Lock		98									
1,200ft	Loc 2	B	51	$151,060	$41,000	$4,066	$196,126	28	n/a	28	192	0.98
		R	55	$112,260	$13,000	$2,716	$127,976	26	n/a	26	179	1.40
	Loc 3	C	57	$197,160	$35,000	$3,996	$236,156	25	15	40	272	1.15
	Loc 4	B	54	$288,430	$0	$2,240	$290,670	27	15	42	282	0.97
		C	54	$289,130	$0	$2,240	$291,370	27	15	42	282	0.97
600ft	Loc 3	C	95	$162,860	$19,000	$2,997	$184,857	15	15	30	203	1.10
	Loc 4	B	95	$238,430	$0	$1,680	$240,110	15	15	30	203	0.84
		C	95	$253,430	$0	$1,680	$255,110	15	15	30	203	0.80
20	Ex. Lock		104									
1,200ft	Loc 2	B	54	$119,900	$25,000	$1,643	$146,543	27	n/a	27	193	1.32
		C	54	$119,900	$38,000	$1,643	$159,543	27	n/a	27	193	1.21
		R	58	$99,200	$11,000	$843	$111,043	25	n/a	25	181	1.63
	Loc 3	B	54	$151,100	$18,000	$1,107	$170,207	26	14	40	291	1.71
		C	54	$156,100	$20,000	$1,107	$177,207	26	14	40	291	1.64
	Loc 4	B	53	$201,230	$18,000	$450	$219,680	27	14	41	296	1.35
		C	53	$197,930	$18,000	$450	$216,380	27	14	41	296	1.37
600ft	Loc 3	B	101	$129,100	$11,000	$830	$140,930	14	14	28	203	1.44
		C	101	$127,100	$11,000	$830	$138,930	14	14	28	203	1.46
	Loc 4	B	101	$170,230	$11,000	$338	$181,568	14	14	28	203	1.12
		C	101	$175,230	$11,000	$338	$186,568	14	14	28	203	1.09
19	Ex. Lock		58									
1,200ft	Loc 3	B	68	$277,050	$18,000	$1,600	$296,650	21	25	46	186	0.63
18	Ex. Lock		99									
1,200ft	Loc 2	B	52	$146,400	$41,000	$600	$188,000	28	n/a	28	192	1.02
		R	55	$110,400	$13,000	$500	$123,900	26	n/a	26	181	1.46
	Loc 3	C	52	$197,250	$35,000	$600	$232,850	28	15	42	290	1.25
	Loc 4	B	50	$280,050	$0	$600	$280,650	29	15	43	297	1.06
		C	50	$272,750	$0	$600	$273,350	29	15	43	297	1.09
600ft	Loc 3	C	98	$162,950	$19,000	$450	$182,400	15	15	29	201	1.10
	Loc 4	B	97	$231,050	$0	$450	$231,500	15	15	29	202	0.87
		C	97	$238,050	$0	$450	$238,500	15	15	29	202	0.85
17	Ex. Lock		105									
1,200ft	Loc 1	C	57	$177,700	$0	$7,500	$185,200	25	14	39	284	1.53
	Loc 2	B	55	$157,100	$41,000	$3,500	$201,600	26	n/a	26	192	0.95
		R	59	$117,600	$13,000	$2,700	$133,300	25	n/a	25	179	1.34
	Loc 3	C	53	$205,000	$35,000	$3,500	$243,500	27	14	41	298	1.22
	Loc 4	B	54	$293,000	$0	$3,500	$296,500	27	14	40	296	1.00
		C	54	$295,700	$0	$3,500	$299,200	27	14	40	296	0.99
600ft	Loc 1	C	104	$169,000	$0	$5,625	$174,625	14	14	27	201	1.15

TABLE 26: SITE-SPECIFIC SUMMARY OF NEW LOCK COSTS, PERFORMANCE, AND COST EFFECTIVENESS												
Lock Site	Lock Location Type		Lock-age Time[1]	Costs w/o Impacts to Nav or Env ($1000)	Estimated Impacts to Nav ($1000) [2]	Site-Specific Env. Cost ($1000) [3]	Total Cost ($1000)	Tow/Day New Lock	Tow/Day Existing Lock	Total Tow/Day Facility	Percent Increase Tow/Day	Percent Increase/ $1 mil
	Loc 3	C	101	$171,700	$19,000	$2,625	$193,325	14	14	28	204	1.06
	Loc 4	B	101	$244,000	$0	$2,625	$246,625	14	14	28	204	0.83
		C	101	$260,000	$0	$2,625	$262,625	14	14	28	204	0.78
16	Ex. Lock		100									
1,200ft	Loc 2	B	51	$162,900	$41,000	$8,100	$212,000	28	n/a	28	198	0.93
		R	55	$124,500	$13,000	$600	$138,100	26	n/a	26	182	1.32
	Loc 3	C	50	$211,700	$35,000	$8,100	$254,800	29	14	43	299	1.17
	Loc 4	B	50	$289,200	$0	$8,100	$297,300	29	14	43	300	1.01
		C	50	$297,900	$0	$8,100	$306,000	29	14	43	300	0.98
600ft	Loc 3	C	98	$177,400	$19,000	$7,695	$204,095	15	14	29	203	0.99
	Loc 4	B	98	$241,200	$0	$7,695	$248,895	15	14	29	203	0.81
		C	98	$262,200	$0	$7,695	$269,895	15	14	29	203	0.75
15	Ex. Lock		111									
1,200ft	Loc 2	B	59	$105,430	$25,000	$600	$131,030	24	n/a	24	187	1.43
		C	59	$98,430	$38,000	$600	$137,030	24	n/a	24	187	1.37
		R	62	$90,630	$11,000	$600	$102,230	23	n/a	23	179	1.75
	Loc 3	B	62	$136,330	$18,000	$600	$154,930	23	13	36	280	1.81
		C	62	$161,330	$20,000	$600	$181,930	23	13	36	280	1.54
600ft	Loc 3	B	109	$113,330	$11,000	$450	$124,780	13	13	26	202	1.62
		C	109	$128,330	$11,000	$450	$139,780	13	13	26	202	1.44
14	Ex. Lock		104									
1,200ft	Loc 2	B	56	$110,330	$25,000	$5,000	$140,330	26	n/a	26	186	1.32
		C	56	$106,330	$38,000	$5,000	$149,330	26	n/a	26	186	1.24
		R	59	$102,530	$11,000	$5,000	$118,530	25	n/a	25	177	1.49
	Loc 4	B	57	$208,200	$18,000	$5,000	$231,200	25	14	39	281	1.22
		C	57	$203,400	$18,000	$5,000	$226,400	25	14	39	281	1.24
600ft	Loc 4	B	103	$188,200	$11,000	$3,750	$202,950	14	14	28	201	0.99
		C	103	$189,700	$11,000	$3,750	$204,450	14	14	28	201	0.98
13	Ex. Lock		92									
1,200ft	Loc 2	B	46	$146,400	$41,000	$1,600	$189,000	31	n/a	31	200	1.06
		R	49	$110,600	$13,000	$1,600	$125,200	29	n/a	29	189	1.51
	Loc 3	C	47	$196,300	$35,000	$1,600	$232,900	30	16	46	296	1.27
	Loc 4	B	47	$280,800	$0	$1,600	$282,400	31	16	46	296	1.05
		C	47	$270,500	$0	$1,600	$272,100	31	16	46	296	1.09
600ft	Loc 3	C	93	$162,300	$19,000	$1,200	$182,500	15	16	31	199	1.09
	Loc 4	B	93	$231,800	$0	$1,200	$233,000	15	16	31	199	0.86
		C	93	$235,800	$0	$1,200	$237,000	15	16	31	199	0.84
12	Ex. Lock		95									
1,200ft	Loc 2	B	47	$146,100	$41,000	$2,500	$189,600	30	n/a	30	200	1.05
		R	50	$121,300	$13,000	$2,500	$136,800	29	n/a	29	189	1.38
	Loc 3	C	47	$198,900	$35,000	$2,500	$236,400	31	15	46	303	1.28
	Loc 4	B	46	$278,600	$0	$2,500	$281,100	31	15	46	304	1.08
		C	46	$275,300	$0	$2,500	$277,800	31	15	46	304	1.09
600ft	Loc 3	C	93	$164,600	$19,000	$1,875	$185,475	15	15	31	202	1.09
	Loc 4	B	93	$230,600	$0	$1,875	$232,475	15	15	31	202	0.87
		C	93	$240,600	$0	$1,875	$242,475	15	15	31	202	0.83
11	Ex. Lock		102									
1,200ft	Loc 2	B	49	$110,100	$41,000	$600	$151,700	29	n/a	29	208	1.37
		R	53	$81,200	$13,000	$500	$94,700	27	n/a	27	193	2.04
	Loc 3	C	49	$203,000	$35,000	$600	$238,600	30	14	44	309	1.29
600ft	Loc 3	C	100	$168,700	$19,000	$450	$188,150	14	14	29	201	1.07

Notes:

[1] Average lockage times (minutes) shown are based on equal percentages of fly, exchange, and turnback lockages. The numbers shown assume turnback approaches occur during the lockage of the previous tow; this may understate overall lockage time slightly.

Cost estimates prepared for these conceptual designs were prepared to the same level of detail as those presented in the *Conceptual Lock Designs Report*. The project element costs are based on 1996 prices and include 25 percent contingencies.

The operations and maintenance costs and the major rehab costs reflect only the incremental life cycle costs for each alternative. Total O&M costs would be $13.1 million for Location 2 locks and $19.0 million for other locations leading to the existence of two locks. However, this $13.1 million is associated with the existence of the existing lock and as such does not represent a with-project cost.

The total costs shown are not all inclusive. They do not include the costs related to system environmental impacts, cultural impacts, and only include rough estimates of impacts to navigation during construction.

[2] Costs for impacts to navigation are estimated based on preliminary system economic model runs, which indicated impact to navigation costs of $12 million per lock associated with the construction of a 2R lock. The costs for the other lock types where then estimated based on relative hours of unavailability. This is at best a rough estimate to provide a general indication of relative level of impacts.
[3] Environmental costs for Locks 20-25, Peoria, and La Grange are the average of the ranges provided in the *Site Specific Habitat Assessment* report. The values for the other sites were estimated based on qualitative analysis of the sites and relative impacts compared to the seven sites which had specific costs identified. The environmental costs associated with the 600-foot locks were estimated at 3/4 the cost of the 1,200-foot lock options but were not looked at in detail.

RESULTS OF SECONDARY SCREENING

The results of this screening are summarized in Table 27. This secondary screening greatly reduced the number of potential lock locations and types. The development of the lower cost 2R lock allowed all other Location 2 lock extensions and 600-foot lock options to be screened out. The IWW sites were the exceptions, where impacts to navigation associated with extending the existing lock lower it in relative performance to Location 1 locks, which can be constructed with minimal impacts to navigation. However, the numerous qualitative limitations lead to the screening of a 600-foot lock option at these sites as well.

Regarding options that would result in the construction of a new 1,200-foot lock in addition to the existing 600-foot lock, Location 1 locks dominated at the four sites where they can be constructed. At all sites, Location 3 locks dominated Location 4 locks. As a result, Location 4 locks were screened out, except at Lock 14 where it is the only option other than extending the existing lock. At Locks 17 and 25, both 1C and 3C locks were carried forward through secondary screening due to considerable unquantified concerns over the environmental impacts associated with Location 1 locks. For the remaining Location 3 and 4 locks, the lowest cost will be carried forward through secondary screening.

TABLE 27: REMAINING LOCK LOCATIONS AND TYPES FOLLOWING SECONDARY SCREENING

Lock and Dam Site	Location Number and Viable Types					
	1	2	3[3]	4[3]	5	6
L/D 11	-	R	C	-	-	-
L/D 12	-	R	C	-	-	-
L/D 13	-	R	C	-	-	-
L/D 14[1]	-	R	-	C	-	-
L/D 15[1]	-	R	B	-	-	-
L/D 16	-	R	C	-	-	-
L/D 17	C	R	C	-	-	-
L/D 18	-	R	C	-	-	-
L/D 19[1]	-	-	B	-	-	-
L/D 20[2]	-	R	B	-	-	-
L/D 21	-	R	C	-	-	-
L/D 22[1]	-	R	C	-	-	-
L/D 24[2]	-	R	C	-	-	-
L/D 25	C	R	C	-	-	-
Peoria	C	R	-	-	-	-
La Grange	C	R	-	-	-	-

[1] These sites have rock foundations. All others (except for note 2) are sand-founded sites (requiring piles).
[2] These sites have mixed foundations; some locations would be rock-founded and some pile-founded.
[3] While only a Type B or C lock is shown, costs and performance evaluations to date have shown these lock options to be essentially equal.

OTHER CONSIDERATIONS

Despite screening out a number of lock locations and types based on cost effectiveness using available cost and performance information, some issues remain that could lead to the eventual recommendation to implement a different location of lock other than the remaining lock options shown above. For example, Location 4 locks were generally rated as a good to best location from a navigation perspective. The main reasons are that it resulted in two locks, presents minimal impacts to existing traffic during construction, and afforded the best opportunity for the optimum approach conditions, especially for downbound tows. There are also similar reasons why a Location 3 lock may be preferred over a lower cost lock extension. A more complete list of potential reasons to select other types includes:

- Ease of Approach and Exit: Locations 3 and 4 in many cases provide significant approach improvement benefits, while some time savings are anticipated for downbound fly and exchange approaches, there is a potential for unquantified safety improvements as well.
- Redundancy of Two Locks: Prevents system shutdowns during necessary repairs and rehabilitation. This also allows for greater increases in traffic growth without the need for further capital infrastructure improvements in the near future.
- Corps Design Criteria/Policy/Standards: Type 2R represents that greatest deviation from current Corps design criteria.
- Uncertainties: Focusing on a location 2 lock extensions present some risks for unanticipated delays during construction and the resulting higher impacts to navigation. Delays due to weather also present greater risks for lock extensions due to reliance on winter closure periods for construction.
- Impacts to Navigation During Construction: The further a new lock location is from the existing lock, the greater the opportunity for virtually continuous, mostly uninterrupted use of the existing lock during construction of the new facility.
- Recreational Craft: Improvement options that provide two locks would allow better accommodation of recreational and other non-commercial vessels. This also improves safety since two chambers would allow various vessels to more easily stay out of each other's way.
- Site-Specific: Some site-specific factors or limitations not fully identified to date may influence the ultimate location (geotechnical, environmental, etc.).

ATTACHMENTS

COST EXTRAPOLATION (BASIC LOCK COST)							
				$ 1000s			
ROCK FOUNDED							
				EST	EST	EST	PER 200'
LOCATION 1		600'	1200'	800'	1000'	400'	
TYPE A		168000	188000	175000	181000	161000	7000
TYPE B		152000	176000	160000	168000	144000	8000
TYPE C		147000	171000	155000	163000	139000	8000
LOCATION 2							
TYPE B			115000				
TYPE C			110000				
LOCATION 3							
TYPE B		109000	131000	116000	124000	102000	7000
TYPE C		96000	122000	105000	113000	87000	9000
LOCATION 4							
TYPE A		176000	206000	186000	196000	166000	10000
TYPE B		163000	184000	170000	177000	156000	7000
TYPE C		154000	165000	158000	161000	150000	4000
LOCATION 5							
TYPE A		195000	234000	208000	221000	182000	13000
TYPE B		159000	186000	168000	177000	150000	9000
TYPE C		146000	161000	151000	156000	141000	5000
COST EXTRAPOLATION (BASIC LOCK COST)							
				$ 1000s			
PILE FOUNDED							
				EST	EST	EST	PER 200'
LOCATION 1		600'	1200'	800'	1000'	400'	
TYPE A		273000	326000	291000	308000	255000	18000
TYPE B		191000	219000	200000	210000	182000	9000
TYPE C		149000	163000	154000	158000	144000	5000
LOCATION 2							
TYPE B			160000				
TYPE C			151000				
LOCATION 3							
TYPE B		172000	200000	181000	191000	163000	9000
TYPE C		160000	194000	171000	183000	149000	11000
LOCATION 4							
TYPE A		339000	373000	350000	362000	328000	11000
TYPE B		232000	283000	249000	266000	215000	17000
TYPE C		217000	248000	227000	238000	207000	10000
LOCATION 5							
TYPE A		287000	342000	305000	324000	269000	18000
TYPE B		205000	246000	219000	232000	191000	14000
TYPE C		181000	227000	196000	212000	166000	15000

ATTACHMENT 1

LOCATION 1							
LOCK AND DAM 25 -- CHAMBER SIZE COMPARISON MATRIX ($000)							
	NON-COM	200 FOOT	400 FOOT	600 FOOT	800 FOOT	1000 FOOT	1200 FOOT
	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER
NON-COM	0	104295	158270	179054	194824	200393	220329
200 FOOT	-104295	0	53975	74759	90529	96098	116034
400 FOOT	-158270	-53975	0	20784	36554	42123	62059
600 FOOT	-179054	-74759	-20784	0	15770	21339	41275
800 FOOT	-194824	-90529	-36554	-15770	0	5569	25505
1000 FOOT	-200393	-96098	-42123	-21339	-5569	0	19936
1200 FOOT	-220329	-116034	-62059	-41275	-25505	-19936	0
PROJECT							
LIFE	5 YEARS	23 YEARS	37 YEARS	46 YEARS	50 YEARS	50 YEARS	50 YEARS

ATTACHMENT 2

LOCATION 2 -- CHAMBER EXTENSIONS					
LOCK AND DAM 25 -- CHAMBER SIZE COMPARISON MATRIX ($000)					
	800 FOOT	1000 FOOT	1200 FOOT		
	EXTENSION	EXTENSION	EXTENSION		
800 EXT	0	4475	208524		
1000 EXT	-4475	0	204049		
1200 EXT	-208524	-204049	0		
PROJECT					
LIFE	3 YEAR	3 YEARS	41 YEARS		
LOCK AND DAM 25 -- 1200 FOOT CHAMBER COMPARISON MATRIX ($000)					
	1200 FOOT	1200 FOOT	1200 FOOT		
	EXTENSION	EXTENSION	EXTENSION		
1200 EXT	0	29297	29592		
1200 LOC 3	-29297	0	295		
1200 LOC 1	-29592	-295	0		
PROJECT					
LIFE	41 YEARS	50 YEARS	50 YEARS		

ATTACHMENT 3

LOCATION 3							
LOCK AND DAM 25 -- CHAMBER SIZE COMPARISON MATRIX ($000)							
	NON-COM	200 FOOT	400 FOOT	600 FOOT	800 FOOT	1000 FOOT	1200 FOOT
	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER
NON-COM	0	59014	137677	167341	177877	192973	220254
200 FOOT	-59014	0	78677	108327	118863	133959	161240
400 FOOT	-137677	-78663	0	29664	40200	55296	82577
600 FOOT	-167341	-108327	-29664	0	10536	25632	52913
800 FOOT	-177877	-118863	-40200	-10536	0	15096	42377
1000 FOOT	-192973	-133959	-55296	-25632	-15096	0	27281
1200 FOOT	-220254	-161240	-82577	-52913	-42377	-27281	0
PROJECT							
LIFE	5 YEARS	15 YEARS	31 YEARS	40 YEARS	45 YEARS	50 YEARS	50 YEARS

ATTACHMENT 4

LOCATION 2					
LOCK AND DAM 22 -- CHAMBER SIZE COMPARISON MATRIX ($000)					
	800 FOOT	1000 FOOT	1200 FOOT		
	EXTENSION	EXTENSION	EXTENSION		
800 EXT	0	1904	103583		
1000 EXT	-1904	0	101679		
1200 EXT	-103583	-101679	0		
PROJECT					
LIFE	1 YEAR	2 YEARS	40 YEARS		
LOCK AND DAM 22 -- 1200 FOOT CHAMBER COMPARIOSN MATRIX ($000)					
	1200 FOOT	1200 FOOT	1200 FOOT		
	EXTENSION	EXTENSION	EXTENSION		
1200 EXT	0	20419	20690		
1200 LOC 3	-20419	0	271		
1200 LOC 4	-20690	-271	0		
PROJECT					
LIFE	40 YEARS	50 YEARS	50 YEARS		

ATTACHMENT 5

LOCATION 3							
LOCK AND DAM 22 -- CHAMBER SIZE COMPARISON MATRIX ($000)							
	NON-COM	200 FOOT	400 FOOT	600 FOOT	800 FOOT	1000 FOOT	1200 FOOT
	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER
NON-COM	0	38544	88937	108710	116015	123395	162705
200 FOOT	-38544	0	50393	70166	77471	84851	124161
400 FOOT	-88937	-50393	0	19773	27078	34458	73768
600 FOOT	-108710	-70166	-19773	0	7305	14685	53995
800 FOOT	-116015	-77471	-27078	-7305	0	7380	46690
1000 FOOT	-123395	-84851	-34458	-14685	-7380	0	39310
1200 FOOT	-162705	-124161	-73768	-53995	-46690	-39310	0
PROJECT							
LIFE	3 YEARS	14 YEARS	29 YEARS	37 YEARS	41 YEARS	46 YEARS	50YEARS

ATTACHMENT 6

LOCATION 4							
LOCK AND DAM 22 -- CHAMBER SIZE COMPARISON MATRIX ($000)							
	NON-COM	200 FOOT	400 FOOT	600 FOOT	800 FOOT	1000 FOOT	1200 FOOT
	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER
NON-COM	0	70592	110232	125813	131967	142713	161402
200 FOOT	-70592	0	39640	55221	61375	72121	90810
400 FOOT	-110232	-39640	0	15581	21735	32481	51170
600 FOOT	-125813	-55221	-15581	0	6154	16900	35589
800 FOOT	-131967	-61375	-21735	-6154	0	10746	29435
1000 FOOT	-142713	-72121	-32481	-16900	-10746	0	18689
1200 FOOT	-161402	-90810	-51170	-35589	-29435	-18689	0
PROJECT							
LIFE	3 YEARS	21 YEARS	35 YEARS	42 YEARS	46 YEARS	50 YEARS	50 YEARS

ATTACHMENT 7

LOCATION 1							
LAGRANGE LOCK AND DAM -- CHAMBER SIZE COMPARISON MATRIX ($000)							
	NON-COM	200 FOOT	400 FOOT	600 FOOT	800 FOOT	1000 FOOT	1200 FOOT
	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER
NON-COM	0	297027	364154	384677	410305	418049	425237
200 FOOT	-297027	0	67127	87650	113278	121022	128210
400 FOOT	-364154	-67127	0	20523	46151	53895	61083
600 FOOT	-384677	-87650	-20523	0	25628	33372	40560
800 FOOT	-410305	-113278	-46151	-25628	0	7744	14932
1000 FOOT	-418049	-121022	-53895	-33372	-7744	0	7188
1200 FOOT	-425237	-128210	-61083	-40560	-14932	-7188	0
PROJECT							
LIFE	4 YEARS	29 YEARS	42 YEARS	49 YEARS	50 YEARS	50 YEARS	50 YEARS

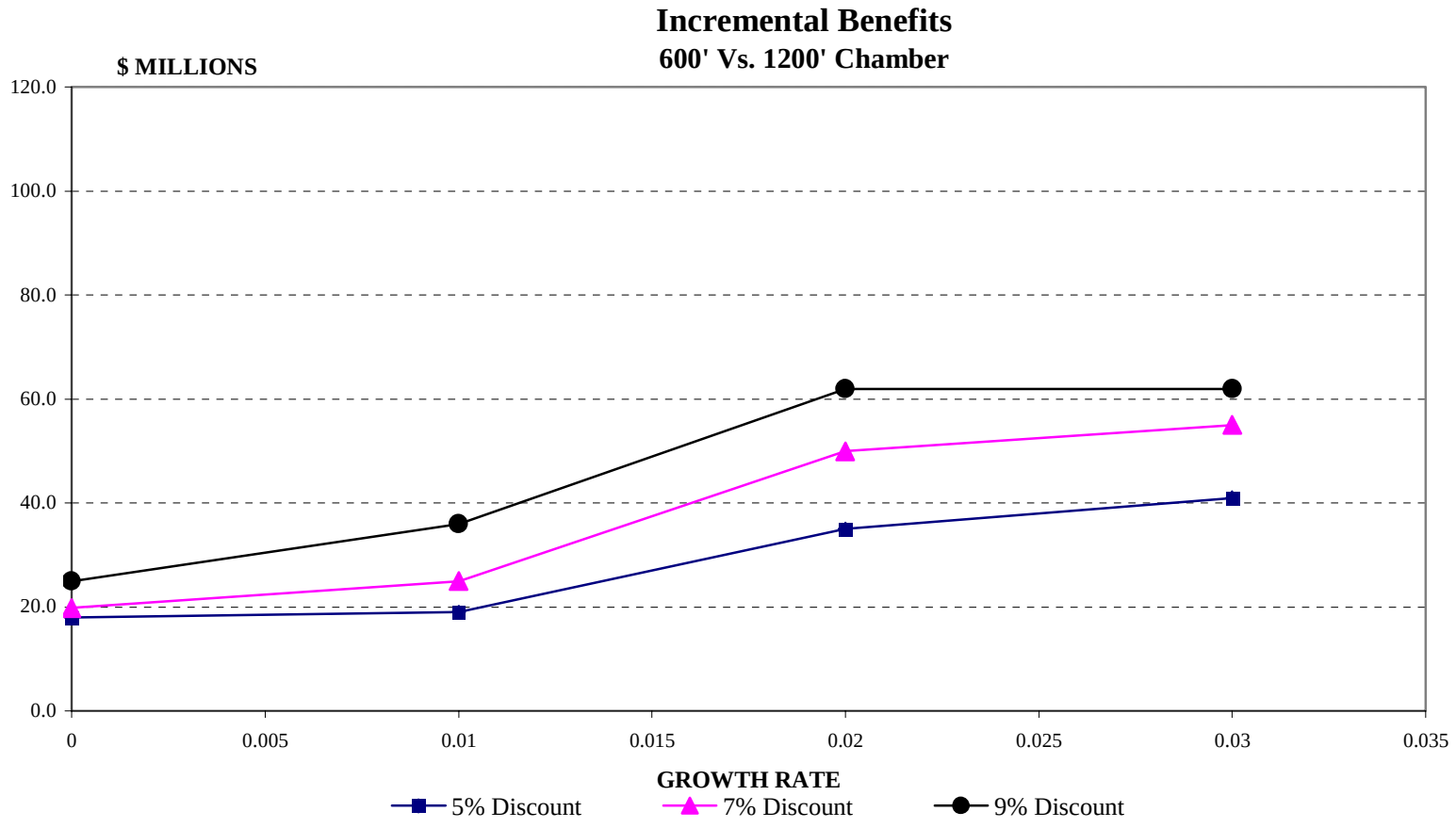
ATTACHMENT 8

	EXTENSION	EXTENSION	EXTENSION			
800 EXT	0	56512	139787			
1000 EXT	-56512	0	83275			
1200 EXT	-139787	-83275	0			
PROJECT						
LIFE	4 YEAR	14 YEARS	40 YEARS			
LAGRANGE LOCK AND DAM -- 1200 FOOT CHAMBER COMPARISON MATRIX ($000)						
	1200 FOOT	1200 FOOT				
	EXTENSION	LOCATION 1				
1200 EXT	0	18928				
1200 LOC 3	-18928	0				
PROJECT						
LIFE	40 YEARS	50 YEARS				

ATTACHMENT 9

LOCATION 3							
LOCK AND DAM 19 -- CHAMBER SIZE COMPARISON MATRIX ($000)							
	NON-COM	200 FOOT	400 FOOT	600 FOOT	800 FOOT	1000 FOOT	1200 FOOT
	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER	CHAMBER
NON-COM	0	206542	261052	290421	304935	326829	361428
200 FOOT	-206542	0	54510	83879	98393	120287	154886
400 FOOT	-261052	-54510	0	29369	43883	65777	100376
600 FOOT	-290421	-83879	-29369	0	14514	36408	71007
800 FOOT	-304935	-98393	-43883	-14514	0	21894	56493
1000 FOOT	-326829	-120287	-65777	-36408	-21894	0	34599
1200 FOOT	-361428	-154886	-100376	-71007	-56493	-34599	0
PROJECT							
LIFE	7 YEARS	24 YEARS	31 YEARS	36 YEARS	39 YEARS	44 YEARS	50 YEARS

ATTACHMENT 10



Based on Lock 22 - Location 4

ATTACHMENT 11